

**PREDICTION OF COMPRESSIVE STRENGTH OF FLY  
ASH-BASED GEOPOLYMER CONCRETE USING  
SUPERVISED MACHINE LEARNING MODELS**

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Final Year Project



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**DEPARTMENT OF CIVIL ENGINEERING  
THE UNIVERSITY OF LAHORE (LAHORE CAMPUS)**

**AUGUST, 2023**

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A final year project submitted in partial fulfilments of the requirements for the award  
of degree of Bachelors of Science in Civil Engineering.

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**DEPARTMENT OF CIVIL ENGINEERING  
THE UNIVERSITY OF LAHORE (LAHORE CAMPUS)  
AUGUST, 2023**

## **DEDICATION**

“I dedicated this effort to my lovely parents, who always pray for my success and their love and affection have always been a source of inspiration for me”

## **DECLARATION**

We declare that this dissertation is our original work, based on original research. Other authors, researchers, and institutions' references have been appropriately cited.

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I begin by offering my heartfelt gratitude to the Almighty Allah. It is through His unwavering presence and wisdom that I have navigated every facet of my journey. His grace, strength, and guidance have been my constant companions. Equally, I acknowledge the profound influence of Prophet Muhammad (ﷺ) whose teachings have illuminated my path.

I extend my sincerest appreciation to my esteemed teachers. Their guidance, engaging discussions, and valuable suggestions have shaped my growth and progress. Their mentorship has been instrumental in shaping my identity today. I am also deeply indebted to my parents. Their unyielding support and unwavering encouragement have sustained me through every challenge. My father's unwavering belief in me and my mother's steadfast confidence in my capabilities have instilled in me the courage to venture beyond limitations.

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Authors

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## ABSTRACT

Fly ash (FA) based geopolymer concrete has lately gained popularity as a low-carbon and sustainable alternative to Portland cement concrete. However, accurate prediction of its compressive strength is still challenging due to the many chemical and physical interactions involved in the geopolymerization process. In this study, three machine learning models backpropagation neural network (BPNN), random forest regression (RFR), and k-nearest neighbors (KNN) were used to estimate the compressive strength of FA-based geopolymer concrete. The models were trained, validated, and tested using a dataset that considered the chemical composition, mix proportions, and pre-curing conditions of the concrete. The coefficient of determination ( $R^2$ ), mean square error (MSE), root mean square error (RMSE), and mean absolute error (MAE) were among the metrics used to evaluate each model's performance. The findings showed that, in comparison to RFR and KNN, which had  $R^2$  values of 0.927 and 0.911, respectively, the BPNN model produced the best results with an  $R^2$  value of 0.948. The coarse aggregate content,  $\text{SiO}_2$  content in FA, and NaOH concentration were shown to have the highest influence on the compressive strength of the FA-based geopolymer concrete, according to the permutation feature important (PFI) index.

## LIST OF ABBREVIATION

ANN	Artificial Neural Network
ML	Machine Learning
PC	Portland Cement
FA	Fly Ash
CA	Coarse Aggregate
FA	Fine Aggregate
W/C	Water to cement ratio
BPNN	Back-Propagation Neural Network
RFR	Random Forest Regressor
KNN	K-Nearest Neighbor
PFI	Permutation Feature Importance
M	Molar
MAE	Mean Absolute Error
MSE	Mean Square Error
RMSE	Root Mean Square Error
SiO <sub>2</sub>	Silicone Dioxide
Al <sub>2</sub> O <sub>3</sub>	Aluminum Trioxide
NaOH	Sodium Hydroxide

## **1. INTRODUCTION**

Concrete is a widely used construction material known for its durability. However, its production contributes significantly to carbon dioxide emissions, exacerbating global warming. As environmental concerns grow, the construction industry is seeking alternatives that conserve natural resources and minimize environmental impacts. One such promising option is geopolymer concrete (GPC), a sustainable material that effectively utilizes industrial waste, particularly fly ash, and serves as a greener alternative to ordinary Portland cement (OPC) concrete.

Globally, Portland cement is the most widely utilized cementing material in concrete production and is associated with considerable energy utilization and elevated carbon dioxide emissions. (Peng & Unluer, 2022). Annually, the world produces 1.6 billion tons of cement, which results in 7% of global carbon emissions, equivalent to 4 billion tons of CO<sub>2</sub>. The generation of emissions in production stems from the utilization of fossil fuels for calcination, production of minerals and transportation, which results in indirect emissions, as well as the direct release of CO<sub>2</sub> during the transformation of calcium carbonate (CaCO<sub>3</sub>) to calcium oxide (CaO). The proportion of indirect and direct CO<sub>2</sub> emissions depends on the production techniques and location, however they both roughly contribute equally (V. M. Malhotra, 2010). The concrete sector consumes

substantial quantities of freshwater, with the yearly requirement for mixing water alone reaching roughly 1 trillion litres. The slower setting and curing speed of concrete that contains a high volume of a mineral admixture can be partially compensated by reducing the water-cement ratio with the assistance of a superplasticizer (V. Malhotra, 1999). A growing trend in the use of Portland cement blends that include cementitious or pozzolanic by-products, such as ground granulated blast-furnace slag and fly ash, is being observed. (Kumar Mehta, n.d.). Fly ash is a byproduct generated from the burning of coal to produce electricity. In 1984, the U.S. produced around 70 million tons of fly ash, of which approximately 7 million tons were utilized in Portland Cement Concrete (PCC) (Gadja & Vangeem, 2001). The usage of supplementary cementitious materials (SCMs) as a replacement for Portland cement (PC) is often limited by proportions. For instance, while fly ash (FA) exhibits pozzolanic effects during various stages of cement hydration, it doesn't play a significant role in early-stage strength development. (V. Malhotra, 1999). The inclusion of FA leads to a decrease in the early rate of hydration and extends the setting time of composite paste. However, this can restrict its utilization in substantial amounts (X. Han et al., 2019).

### **1.1.BACKGROUND**

The production of OPC, the main component of traditional concrete, is expected to increase substantially in the coming years due to global infrastructure development. This surge in OPC production poses serious environmental and health hazards, as cement production accounts for about 7% of total greenhouse gas emissions. The excessive release of carbon dioxide into the atmosphere may lead to a significant rise in Earth's temperature. Furthermore, the disposal of industrial wastes like fly ash and other by-products creates challenges in terms of land usage and environmental impact.

In response to these challenges, researchers have explored the potential of geopolymer concrete as an eco-friendly alternative to OPC. Geopolymer concrete is a novel material that utilizes alkali activation of aluminosilicate-rich materials, including fly ash, metakaolin, red mud, and ground granulated blast furnace slag. Among these materials, fly ash stands out due to its abundance, low cost, and high potential for making geopolymers.

## **1.2.SIGNIFICANCE**

This research brings significant benefits to both the construction industry and the research community. It introduces a novel method to address challenges in civil engineering, offering a fresh approach to problem-solving. By embracing these innovative methods, the construction industry stands to gain in terms of time and cost savings, which would otherwise be expended using conventional approaches. Moreover, the research community can expand their knowledge and expertise by exploring and refining these new methods, fostering advancements in the field of civil engineering.

The use of geopolymer concrete can significantly reduce CO<sub>2</sub> emissions compared to OPC concrete. In fact, studies suggest that GPC contributes about 20–50% lower CO<sub>2</sub> gas emissions. Notably, the carbon footprint of geopolymer concrete has garnered attention in sustainable construction practices. Geopolymer concrete's capability to reduce CO<sub>2</sub> emissions by approximately 80% compared to conventional cement industries makes it a compelling choice for reducing the environmental impact of construction activities.

The use of fly ash and blast furnace slag in the production of other construction materials would both solve an environmental issue and lead to the creation of new, high-performing materials. The search for more environmentally friendly cement-based

binders through the process of alkali activation is a commonly studied method for completely replacing Portland cement (Puertas et al., n.d.). Fly ash has been utilized as a substitute material for the creation of geopolymer, a novel binding agent with similar appearance, reactivity, and characteristics as hydrated cement.

### **1.3.SCOPE AND LIMITATION**

Geopolymer concrete exhibits high early compressive strength, low shrinkage, significant resistance to creep, and good performance in acidic environments. However, its long-term durability remains an area requiring further research. The service life and durability of concrete structures largely depend on material transport properties, such as permeability, sorptivity, and diffusivity, which require in-depth investigation for geopolymer concrete.

As the world becomes more conscious of the need for sustainable practices in civil engineering, geopolymer concrete has emerged as a promising solution. Its utilization of waste materials and reduced CO<sub>2</sub> emissions make it a sustainable alternative to traditional concrete. However, further research and development are necessary to fully understand its long-term durability and to overcome any challenges associated with its application.

### **1.4.THESIS ORGANIZATION**

The thesis comprises five comprehensive chapters, each delving into distinct aspects of the subject matter, thereby providing a thorough and insightful examination of the topic.

#### **1.4.1. CHAPTER 1 – INTRODUCTION**

This pivotal chapter presents the readers with essential background information pertaining to the theme, followed by a clear statement of the problem at hand. Additionally, it highlights the significance of the problem in the context of the



construction industry. The scope and limitations of the study are carefully delineated to provide a concise framework. Lastly, the chapter outlines the organization of the thesis, guiding readers through the subsequent chapters.

#### **1.4.2. CHAPTER 2 - LITERATURE REVIEW**

In this chapter, an extensive review of relevant literature on various aspects is provided, including the classification of concrete, factors influencing compressive strength development at different ages, and predictive methods for compressive strength. An in-depth exploration of Artificial Neural System (ANN) and Regression Models, its training procedure is presented, along with a discussion of the ANN model and experimental results, and the compatibility and achievements derived from this research.

#### **1.4.3. CHAPTER 3 - RESEARCH METHODOLOGY**

This chapter offers valuable insights into the research approach adopted, the sources and nature of data used, and the utilization of Artificial Neural Network (ANN) and Regression Models as a vital tool in the analysis. Detailed information is provided regarding data processing and analyzing techniques, as well as the step-by-step process of modeling an ANN and Regression for the research.

#### **1.4.4. CHAPTER 4 - RESULTS AND DISCUSSION**

Within this chapter, a detailed discussion is provided, primarily focusing on the experimental results acquired from 11 references, alongside the predictions generated by the ANN model for concrete compressive strength. An in-depth analysis of these findings serves to shed light on their implications for the field of civil engineering.

**1.4.5. CHAPTER 5 - CONCLUSIONS AND RECOMMENDATIONS**

As the final chapter, this section offers a comprehensive conclusion, summarizing the research outcomes gained through ANN modeling. Based on these results, informed and practical recommendations are provided, adding to the existing body of knowledge and offering valuable insights for future research and application.

By adopting a meticulous and structured approach, this thesis contributes significantly to the construction industry and research community alike, paving the way for the advancement of knowledge and innovative problem-solving techniques in civil engineering

## **2. LITERATURE REVIEW**

### **2.1.FLY ASH**

Fly ash is a fine, powdery substance that results from burning pulverized coal in electric power plants. When coal is burned, its mineral impurities, mainly silica, alumina, and iron, react with lime during combustion, forming small spherical particles known as fly ash. These particles are then captured from the flue gas by pollution control devices like electrostatic precipitators or baghouses.

As a pozzolanic material, fly ash exhibits cementitious properties when combined with water and an alkaline activator, such as sodium hydroxide or potassium hydroxide. This characteristic makes fly ash an essential component in the production of cement and concrete. By incorporating fly ash as a partial replacement for cement in concrete, the environmental impact of concrete manufacturing can be reduced. This reduction is achieved by lowering the demand for clinker production, a key ingredient in cement manufacturing and a major source of carbon dioxide emissions. Moreover, the addition of fly ash in concrete can enhance specific properties, including workability, durability, and resistance to certain chemical attacks.

Due to its utilization of waste material from power generation and its contribution to minimizing the consumption of natural resources, fly ash is regarded as an environmentally sustainable material. Its versatility and positive environmental

attributes make it a valuable resource with widespread applications in the construction industry and other related fields. Fly ash has been utilized as a substitute material for the creation of geopolymer, a novel binding agent with similar appearance, reactivity, and characteristics as hydrated cement. Essentially, geopolymer is formed through the activation of aluminosilicate materials with alkalis. Alkali activation is achieved by adding sodium hydroxide (NaOH), potassium hydroxide (KOH), sodium silicate ( $\text{Na}_2\text{SiO}_3$ ) or potassium silicate ( $\text{K}_2\text{SiO}_3$ ) individually or in combination with fly ash. This process, known as geopolymerization, can take place at room temperature or slightly elevated temperatures (typically below  $100^\circ\text{C}$ ) and most importantly, with minimal  $\text{CO}_2$  emissions. Geopolymers possess advantageous mechanical and durability properties due to their distinct chemical structure, and are eco-friendly as they utilize industrial waste as a binding material in contrast to mixes based on Portland cement (PC). Fly ash geopolymer cement is a cost-effective, low-carbon, low-energy alternative to ordinary Portland cement.

## **2.2.PROPERTIES OF FLY ASH**

Fly ash geopolymers have a better ability to adsorb and stabilize toxic metals compared to ordinary Portland cement (OPC) and can be utilized as a sealant for storing  $\text{CO}_2$ , however with lower permeability when compared to typical sealants.(Zhuang et al., 2016). Geopolymer mortar and concrete exhibit comparable strength, texture, and appearance, and offer similar mechanical characteristics (Chindaprasirt et al., 2011). The interlinked structure of geopolymers results in high compressive strengths. Due to the microstructure of their reaction products, geopolymers and Alkali-Activated Binders (AAB) exhibit good to excellent resistance to chemical degradation such as sulphate attack, acid attack, and seawater exposure (Soutsos et al., 2016). Fly ash also

demonstrates improved durability, including resistance to high temperatures (Kong & Sanjayan, 2010) and chloride penetration (Chindaprasirt & Chalee, 2014).

### **2.3.FACTORS AFFECTING PROPERTIES OF FLY ASH**

In the geopolymerization process, fly ash plays a vital role as a raw material. However, several factors can influence its behaviour and properties during this process.

#### **2.3.1. CHEMICAL ACTIVATORS**

The presence of a chemical activator or alkali activator solution is crucial for initiating the geopolymerization of fly ash. A strong alkaline medium, typically containing NaOH, enhances the surface hydrolysis of the aluminosilicate particles in the fly ash, which is essential for the reaction to occur.

#### **2.3.2. CONCENTRATION OF CHEMICAL ACTIVATORS**

The concentration of the chemical activator has a pronounced effect on the mechanical properties of the geopolymers formed. Increasing the concentration of chemical activators, especially NaOH, has been shown in research papers to lead to enhanced compressive strength in geopolymers.

#### **2.3.3. MULTI-COMPOUND CHEMICAL ACTIVATORS**

The most effective chemical activators for improving mechanical strength in geopolymers are those containing  $\text{Na}_2\text{-SiO}_3/\text{NaOH}$ . Incorporating these multi-compound activators into the geopolymer matrix contributes to the desired strength enhancement.

#### **2.3.4. HEAT CURING**

Applying heat curing is crucial to expedite the initial strength growth of fly ash-based geopolymer concrete. Both the duration and temperature of this process play pivotal

roles. Generally, the recommended optimal heat curing conditions encompass a maximum period of 24 hours, coupled with a temperature span ranging from 50°C to 90°C. These conditions have proven advantageous for fostering both short-term and long-term strength enhancement, as well as ensuring the durability and steadfastness of geopolymer concrete.

**Liquid Alkaline/Ash Ratio:** The ratio of liquid alkaline to fly ash in the mixture holds sway over the workability of fresh geopolymer blends. Elevating the liquid alkaline/ash ratio typically yields improved workability.

### **2.3.5. Na<sub>2</sub>-SiO<sub>3</sub>/NaOH RATIO**

The ratio of Na<sub>2</sub>-SiO<sub>3</sub> to NaOH in the mixture affects the workability of fresh geopolymer mixtures. Higher Na<sub>2</sub>-SiO<sub>3</sub>/NaOH ratios can reduce workability due to the increased viscosity of Na<sub>2</sub>SiO<sub>3</sub>.

### **2.3.6. NAOH CONCENTRATION**

Higher concentrations of NaOH in the geopolymer mixture can lead to longer setting times. It is crucial to consider the appropriate concentration to achieve the desired setting time and performance.

### **2.3.7. SHRINKAGE**

The shrinkage of geopolymer concrete is closely related to its corresponding strength. Lower strength development in geopolymers is associated with increased shrinkage.

Lastly, understanding the factors that affect fly ash during the geopolymerization process is vital for optimizing the properties and performance of geopolymers. Careful consideration of chemical activators, heat curing conditions, and mixing ratios can lead to geopolymers with enhanced mechanical strength, workability, and reduced shrinkage.

## **2.4.NEURAL NETWORKS**

Neural networks are a collection of strong machine learning algorithms that have been influenced by the structure and operation of the human brain. These artificial neural networks are made up of linked nodes, or neurons, that are arranged in layers. The input layer receives raw data, which is subsequently passed on to hidden layers, where each neuron conducts mathematical computations on the data. During training, the degree of interconnections between neurons, indicated by weights, is modified to learn patterns and correlations in the data. The last layer, known as the output layer, generates predictions or classification based on the previously acquired patterns. Neural networks have demonstrated extraordinary ability in tasks like as picture and audio identification, processing natural languages, and strategic game play. With advances in machine learning and an abundance of datasets and computational power, neural networks tend to transform different sectors and drive artificial intelligence forward. The most widely used ML methods can be grouped into four main categories: ANN, SVM, decision trees, and evolutionary algorithms. Artificial Neural Networks (ANN) operate by transmitting information through interconnected neurons, where each information is weighted based on its significance.

## **2.5.ARTIFICIAL NEURAL NETWORK**

An Artificial neural Network (ANN) is an algorithmic framework that tries to imitate the structure and operation of biological brain networks. There are two types of ANN applications: classification models and regression models. Researchers have extensively investigated the application of ANN for forecasting concrete compressive strength. In terms of accuracy, they discovered that ANN models trump standard regression models. The multilayer perceptron (MLP) is the most widely utilized of the different ANN models. The MLP model is made up of an input layer with sensory input nodes, one or

more hidden layers for calculation, and an output layer with a single node representing the compressive strength of concrete. The most extensively used and successful learning strategy for training the MLP model is the back-propagation (BP) algorithm. Equations (1) and (2) mathematically explain the activation mechanism for each neuron in the MLP.

$$net_k = \sum w_{kj} o_j \quad (1)$$

$$y_k = f(net_k) \quad (2)$$

The neurons in the preceding layer (denoted as  $j$ ) impact the activity of neuron  $k$  (denoted as  $net_k$ ) in the context of the neural network model. The weight  $w_{kj}$  represents the link between neurons  $k$  and  $j$ . Neuron  $j$ 's output is indicated as  $o_j$ , while neuron  $k$ 's ultimate output is commonly computed using sigmoid or logistical transfer functions and is labeled as  $y_k$ . The illustration of ANN is depicted in Fig.1.

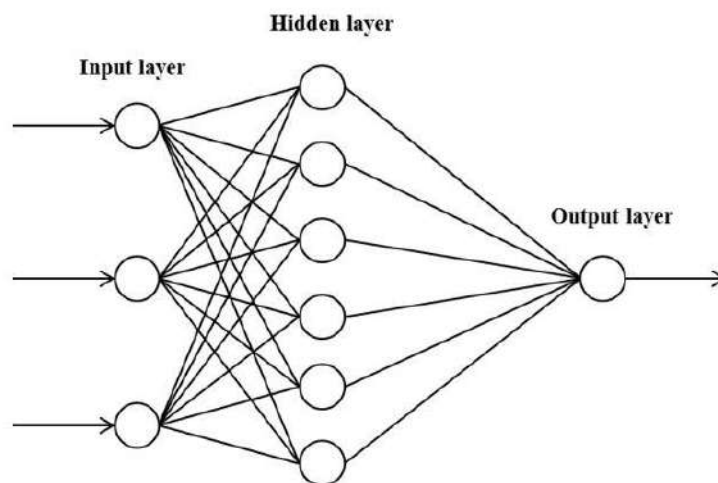


Fig 1: Structure of ANN with one hidden layer (Jin et al., 2016)

## 2.6. NON-LINEAR REGRESSOR

Non-linear regression is a type of regression analysis used in artificial intelligence methods to describe connections between variables when data does not follow a linear pattern. Non-linear regression, as opposed to linear regression, which assumes a linear



connection between the independent and dependent variables, allows for more complicated and flexible interactions.

A non-linear function represents the connection between the independent factors and the dependent variable in non-linear regression. The function's shape might change based on the task and the type of the data. Polynomial functions, exponential functions, logarithmic functions, and trigonometric functions are common examples of non-linear functions used in regression. Non-linear regression is especially beneficial when working with data that contains complex patterns and connections, or where a linear model is unable to fully reflect the underlying trends. It is extensively used in AI systems for jobs like as demographic prediction.

$$y = f(x, \beta) + \varepsilon \quad (3)$$

Where:

- $y$  is the dependent variable (the predicted output).
- $x$  is the independent variable (the input or predictor variable).
- $f(x, \beta)$  is the non-linear function with parameters  $\beta$  that captures the relationship between  $x$  and  $y$ .
- $\varepsilon$  represents the error term, accounting for the difference between the predicted value and the actual value in the data.

## **2.7.SUPPORT VECTOR MACHINE**

SVM is an algorithm that is used for supervised learning problems. It is most commonly used for classification issues in which the aim is to categorize data into multiple classes or groups. SVM determines the appropriate hyperplane (a decision boundary) for separating data points into their respective classes with the greatest margin. It seeks to

minimize the distance between the closest data points of distinct classes, allowing for greater generalization and performance on fresh data.

The k-nearest neighbour (kNN) algorithm is a traditional non-parametric technique (Cover & Hart, 1967). The kNN algorithm is designed to find k-nearest neighbours of a query and assign a class label to the query through the majority voting rule. Due to its simplicity, effectiveness, and intuitiveness, it is one of the most commonly used algorithms in computer programming today (Jiang et al., 2012; Wu et al., 2007). The kNN algorithm is a non-parametric classification method that does not need a training process. Particularly, it does not require prior knowledge about the statistical properties of the training instances (Li et al., 2008; Pan et al., 2020). The illustration of SVM is depicted in Fig.2.

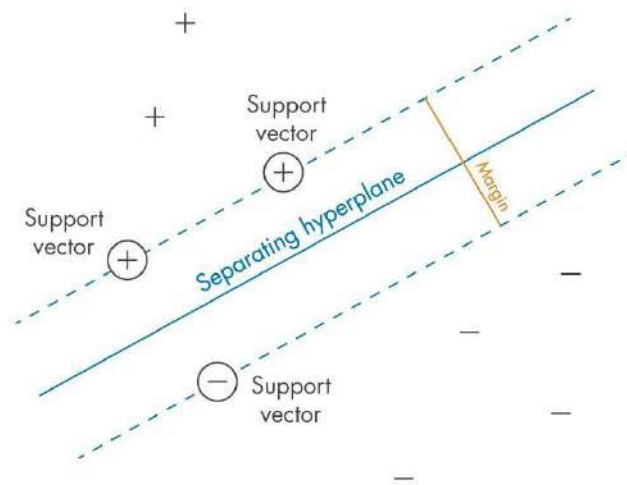


Fig 2: Depiction of SVM Model (Dener et al., 2022)

The functional relationship between one or more independent variables and the response variable is explained by the following model:

$$y(x) = w^T \phi(x) + b \quad (4)$$

where  $x \in R$ ,  $y \in R$ , and  $\phi(x): R^n$  is the process of mapping to higher dimensional feature space.

## **2.8.APPLICATION OF NEURAL NETWORKS IN CIVIL ENGINEERING**

Over the last five years, there has been a boom in interest among academics in the application of artificial neural networks (ANN) in the subject of Civil Engineering. Because of its ability to handle many issues and complexity in the area, the application of ANN in Civil Engineering has piqued the interest of several academics. ANNs have been investigated for a variety of applications, including structural analysis, material characteristics prediction, construction process optimization, and predictive decision-making in civil engineering projects. The capacity of artificial neural networks (ANNs) to learn from data as well as adapt to complicated patterns has proven useful in tackling real-world engineering challenges. This increased interest in ANN in Civil Engineering is likely to fuel more developments and improvements in the industry, resulting in more efficient solutions.

A number of researchers have applied neural networks to forecast compressive strengths, shear strength of RC Deep Beams, and investigate the impact of various parameters on the outcomes. As a result, this approach has proven to be time-efficient and cost-effective. (H. Zhang et al., 2020) employed two models to forecast the residual compressive strength of geopolymer concrete after being subjected to varying temperatures. These two models were created utilizing a Gaussian-centered mathematical framework, with the coefficients established by analyzing experimental outcomes using MATLAB software. Notably, it was noticed that the actual compressive strengths tended to surpass the predicted strengths(H. Zhang et al., 2020).

The utilization of machine learning models has gained extensive adoption as a potent means to forecast the mechanical characteristics of concrete. These models typically work with sizable datasets, which are commonly partitioned into distinct training, validation, and testing segments. The training subset facilitates model training, whereas

the validation dataset ensures impartial assessment of the model's alignment with the training data. It also safeguards against overfitting by stopping the training process if errors start to rise. Finally, the model is applied to the testing data to evaluate its predictive performance.

In the study by Naderpour et al. (Naderpour et al., 2018), the backpropagation artificial neural network was utilized. The study was conducted to develop an Artificial Neural Network (ANN) for evaluating the strength properties of recycled aggregate concrete. The ANN was based on predetermined input variables that were considered key in determining the strength properties. Later a study conducted by Asteris et al. (Asteris & Kolovos, 2019) used Artificial Neural Network (ANN) to predict the compressive strength of self-compacting concrete (SCC). The model was trained using the Levenberg-Marquardt algorithm and the results showed that Backpropagation Artificial Neural Network (BPNN) was effective in accurately predicting the compressive strength of SCC. The sensitivity analysis conducted further revealed that viscosity-modifying admixtures had the greatest impact on the compressive strength of SCC. Random Forest has been adopted as a forecasting tool in several studies. The technique entails combining multiple decision trees, each generated from a different training set through the bagging method (ben Chaabene et al., 2020). The bagging method, commonly referred to as bootstrap aggregation, is a machine learning technique that involves generating new datasets from the original data by randomly resampling it, and using these datasets to train individual base predictors independently. In the final step, the predictions from each base predictor are averaged to produce the final prediction. This method has been applied by various researchers in predicting the mechanical properties of concrete. In the study by Han et al. (Han et al., 2019), Random Forest (RF) was used to predict the compressive strength of High-Performance Concrete (HPC).

Another study by Mangalathu et al. (Mangalathu & Jeon, 2018) utilized the same RF method to forecast the shear strength of Reinforced Concrete (RC) beam-column joints and obtained good agreement in results. Zhang et al. (J. Zhang et al., 2019a) also used RF, but combined it with a beetle antennae search (BAS) algorithm, to evaluate the uniaxial compressive strength of Self-Compacting Concrete (SCC). The BAS algorithm was based on the behavior of beetles as they search for a location with a higher concentration of odor using their antennae. These studies demonstrate that RF can produce accurate predictions for the mechanical strength of concrete. These approaches make things economic in terms of both cost and time.

As Neural Networks in civil engineering provide efficient solutions for predicting material properties, structural behaviour, and complex interactions. Their adaptability to diverse data sets saves time and costs while enhancing decision-making and improving the overall performance of civil engineering projects.

### **3. RESEARCH METHODOLOGY**

#### **3.1.PREDICTION OF COMPRESSION STRENGTH**

The study focuses on predicting the compressive strength of geopolymer concrete using three machine-learning algorithms. It will analyse the chemical composition, mix proportions, and pre-curing conditions of geopolymer concrete to determine the most accurate algorithm for predicting its 28-day compressive strength. The study aims to fill a research gap and provide insights into using machine learning algorithms to predict geopolymer concrete strength.

#### **3.2.MACHINE LEARNING APPROACHES**

##### **3.2.1. MACHINE LEARNING MODELS**

###### **3.2.1.1. BPNN**

Backpropagation Neural Network (BPNN) has gained extensive use as a learning algorithm within multilayer neural networks. Initially introduced by Paul Werbos in 1974 and subsequently rediscovered by Rumelhart and Parker, BPNN stands as the favoured, efficient, and accessible model for intricate, layered networks. Backpropagation is a form of supervised learning rooted in the gradient descent approach. It aims to minimize network errors by descending along the gradient of the error curve. (Hamid et al., 2011; Khalil et al., n.d.). Because of the weight error correct

rules, the supervised learning model is the most popular. BPNN only needs one hidden layer to meet the requirement of prediction accuracy (Peng & Unluer, 2022). The structure of this neural network consists of input sets and one or more layers of neurons. Neurons are connected via layers of parallel neurons except for the special input  $X_0$  which partially represents each neuron. Each neuron in one layer is linked to all neurons in the following layer. Hidden layers are the last layer that creates the model's output and any layers that come before it. The only role of inputs is to feed input patterns into the rest of the network; they do no computing (Sapna, 2012).

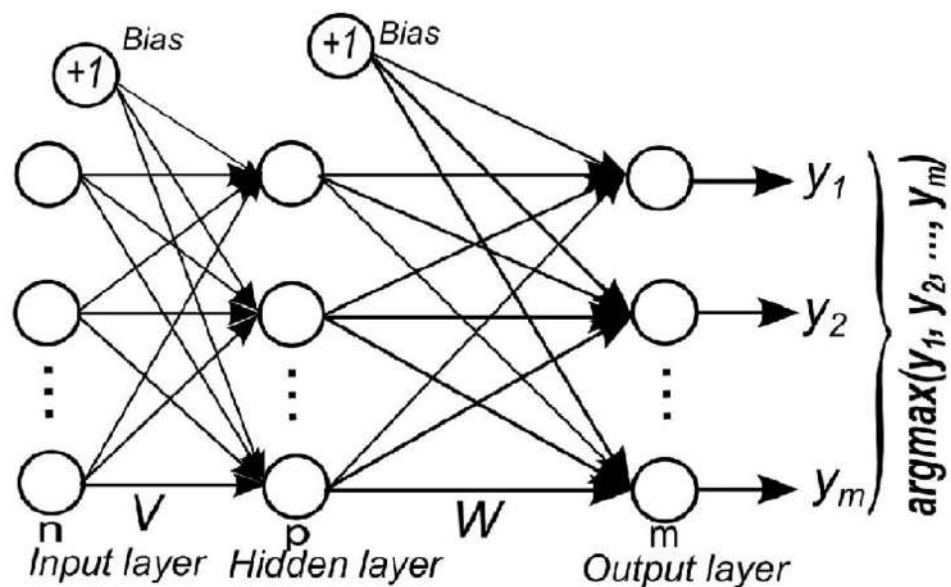


Fig 3: Illustration of BPNN (Yeresime et al., 2014)

### 3.2.1.2. RFR

RFR was proposed by Breiman. Random forests suggest adding an extra layer of randomization to bagging. Random forests are a useful method for making predictions, and they are less likely to overfit due to the Law of Large Numbers. By incorporating appropriate forms of randomness, they can accurately classify and predict regression outcomes. RFR creates By employing distinct bootstrap data samples for each tree, random forests alter the construction process of classification or regression trees. While

conventional trees divide nodes using the optimal split across all variables, random forests introduce a unique approach. At each node, the division is executed using the superior choice from a subset of predictors selected randomly. This seemingly unconventional technique yields impressive performance, surpassing several other classifiers like discriminant analysis, support vector machines, and neural networks. Furthermore, it exhibits resilience against overfitting as outlined by Breiman in 2001. Moreover, its simplicity shines through, as it only necessitates two parameters (the count of variables in the random subset at each node and the number of trees in the forest) and generally showcases robustness to their values (Liaw & Wiener, 2002). The data are split into several points for each input variable, and the Sum of Square Error (SSE) is computed for both the predicted and actual values at each point. Subsequently, this node's least SSE value is obtained (J. Zhang et al., 2019b).

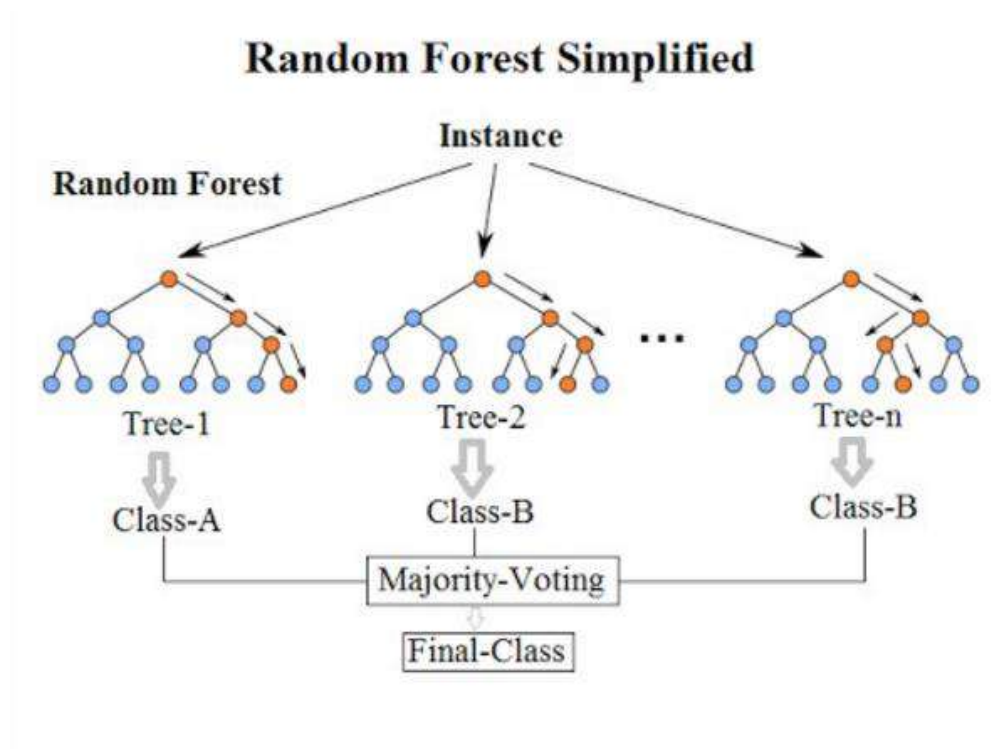


Fig 4: Illustration of RFR (Raja & Fokoué, 2019)



### 3.2.1.3. KNN

The k-nearest neighbor (kNN) algorithm is a traditional non-parametric technique (Cover & Hart, 1967). The kNN algorithm is formulated to identify the k-closest neighbors of a given query and designate a class label to the query using the principle of majority voting. Its straightforwardness, efficiency, and natural approach contribute to its effectiveness. It is one of the most commonly used algorithms in computer programming today (Jiang et al., 2012; Wu et al., 2007). The kNN algorithm is a non-parametric classification technique that operates without the need for a training phase. Specifically, it doesn't necessitate prior understanding of the statistical characteristics of the training examples (Li et al., 2008; Pan et al., 2020).

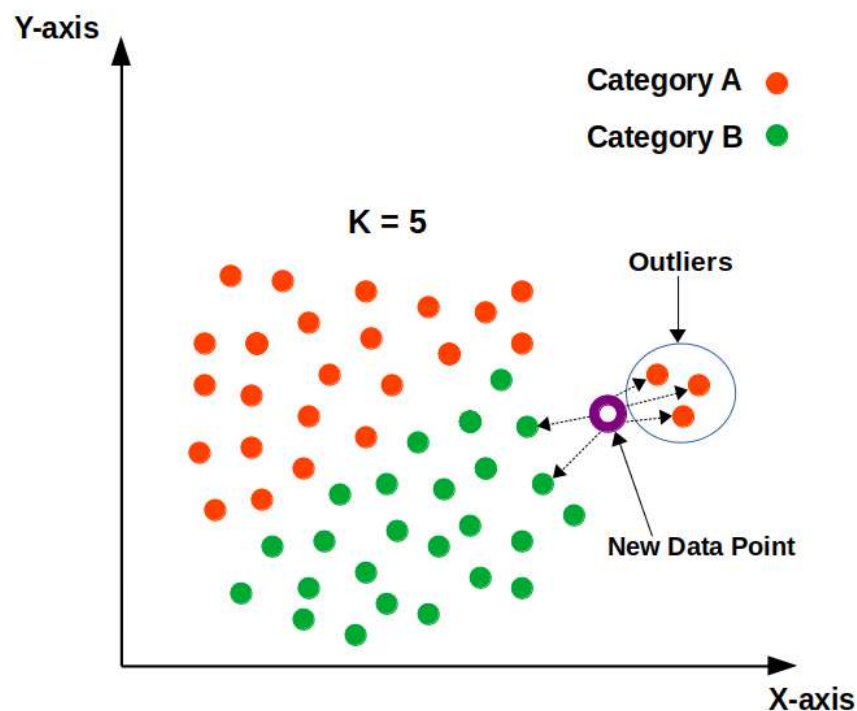
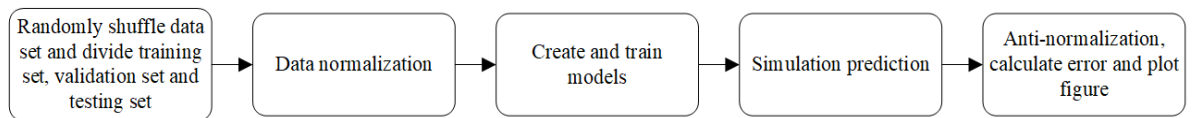


Fig 5: Illustration of KNN (Shivam Sharma, 2021)

## 3.2.2. TRAINING AND EVALUATING MACHINE LEARNING MODELS

Fig.6 shows the combined calculation process of all three machine learning algorithms. Firstly, the collected data were arranged in random order. BPNN parameters were

established as they control the balance between gradient descent and the LM algorithm. The Levenberg-Marquardt algorithm is a method specifically designed for minimizing functions that consist of sums of squares of nonlinear functions. It is particularly suitable for training neural networks. It is well-suited for training neural networks, and despite requiring many computations, it appears to be the quickest training algorithm for neural networks with moderate numbers of parameters.



*Fig 6: Process of Machine Learning Process*

In particular, when it comes to multilayer networks used for function approximation that have up to a few hundred weights and biases, the Levenberg-Marquardt algorithm is generally the fastest training method (Demuth & De Jesús, n.d.). BPNN was trained on MATLAB 2021a while the other two models RFR and KNN were trained on PYTHON. The hyperparameters that were used in training the models are presented in Table I. For each of the models, different hyperparameters were used. Each of the models has its parameters. To make sure that the data was effectively divided and analysed, all the algorithms were used to divide the data into three different sets: a training set, a validation set, and a testing set. These sets were made up of 70%, 15%, and 15% of the total data, respectively. This division was carried out to ensure that the data was thoroughly evaluated and that all the models were properly trained and tested. The LM algorithm is a commonly used optimization method for training Backpropagation Neural Networks (BPNN). It is widely used in various fields and is considered a standard technique for non-linear least-squares problems. The LM algorithm can be perceived as a fusion of the steepest descent and Gauss-Newton techniques. It mimics the behavior of a steepest descent method when the current solution is considerably

distant from the correct solution. Conversely, it emulates the Gauss-Newton approach when the current solution is in proximity to the accurate solution. This algorithm has several advantages over other BPNN algorithms, including good convergence, training precision, and efficiency. Additionally, it is the fastest method for training moderate sized feedforward neural networks (Demuth & Beale, 1992; Hagan & Menhaj, 1994; Yue, 2010). Maximum validation failures were set to 6 to make it stop early as early stopping is a well-established technique in machine learning that can save computational resources and time while still achieving optimal performance (Orr & Müller, 1998). The training stopped when the validation error has increased more than maximum validation failures, at that time the number of iterations were 15 and the value of gradient was 1.37. In T The neural network's number of hidden layers was established as 19, based on the fact that it attained the highest degree of goodness of fit and the lowest calculation error. The goodness of fit reached the maximum and the calculation error was the least. The sigmoid function was used as the activation function of BPNN. The sigmoid function is a popular activation function utilized in neural networks because of its key attributes such as differentiability and smoothness, which play a crucial role in facilitating the back-propagation algorithm (Kros et al., 2006). The sigmoid function is shown in Eq.5.

$$s(x) = \frac{1}{1+e^{-x}} \quad (5)$$

The data was scaled using the scikit-learn library in Python for the machine-learning models RFR and KNN. Standard Scaler Standardize features by removing the mean and scaling to unit variance. The standard score of a sample  $x$  is calculated as:

$$z = \frac{(x-u)}{s} \quad (6)$$

where  $u$  is the mean of the training samples or zero. Centring and scaling happen independently on each feature by computing the relevant statistics on the samples in

the training set. In KNN, the weights were set to default as uniform showing all points were weighted equally in the neighborhood. In RFR, Bootstrap is set to True because it is a useful technique for improving performance. Bootstrapping can improve the performance of the model by reducing the variance and overfitting, and robustness of the model, and for making efficient use of the available data (Breiman, 2001; Geurts et al., 2006). To assess the inconsistency between the predicted value and the measured value for a particular sample, error, and error percentage were used as shown in equation Equ.7 and Equ.8, where  $y_i'$  is the predicted value and  $y_i$  is the actual value.

$$\text{Error} = y_i' - y_i \quad (7)$$

$$\text{Error Percentage} = \frac{y_i' - y_i}{y_i} \quad (8)$$

As the above equation was limited to just one simple sample, to evaluate the entire prediction results, several analytical approaches were used which led to the evaluation of the models. In this evaluation, the coefficient of determination ( $R^2$ ), mean square error ( $MSE$ ), root mean square error ( $RMSE$ ) and mean absolute error ( $MAE$ ). These parameters can compare the performance of various models, quantify the prediction accuracy of a single algorithm, and determine which model is best suited and most applicable to the given database. The expressions for these parameters are shown in equations Equ.9, 10, 11 and 12.

$$R^2 = 1 - \frac{\sum_{i=1}^n (\hat{y}_i - y_i)^2}{\sum_{i=1}^n (\bar{y} - y_i)^2} \quad (9)$$

$$MSE = \frac{1}{n} \sum_{i=1}^n (\hat{y}_i - y_i)^2 \quad (10)$$

$$RMSE = \sqrt{\frac{1}{n} \sum_{i=1}^n (\hat{y}_i - y_i)^2} \quad (11)$$

$$MAE = \frac{1}{n} \sum_{i=1}^n |\hat{y}_i - y_i| \quad (12)$$

### 3.2.2.1. HYPERPARAMETERS OF MACHINE LEARNING MODELS

Table I: Hyperparameters of machine learning models

KNN	N_Neighbours	2
	Weights	Uniform
	Metric	Minkowski
BPNN	Max no of epox	1000
	Max validation failures	6
	Min performance gradient	1e -7
	Hidden layer numbers	19
	Algorithm	Levenberg-Marquardt
RFR	Number of trees	100
	Minimum samples split	2
	Minimum sample leaf	1
	Bootstrap	True

### 3.2.3. PLATFORMS USED IN TRAINING MACHINE LEARNING MODELS

#### 3.2.3.1. MATLAB

MATLAB is a proprietary multi-paradigm programming language and numeric computing environment developed by MathWorks. MATLAB allows matrix manipulations, plotting of functions and data, implementation of algorithms, creation of user interfaces, and interfacing with programs written in other languages. MATLAB is a closed-source programming language and numeric computation platform created by MathWorks. It supports multiple programming styles and offers tools for working with matrices, visualizing functions and data, executing algorithms, building user interfaces, and integrating with software written in different programming languages.

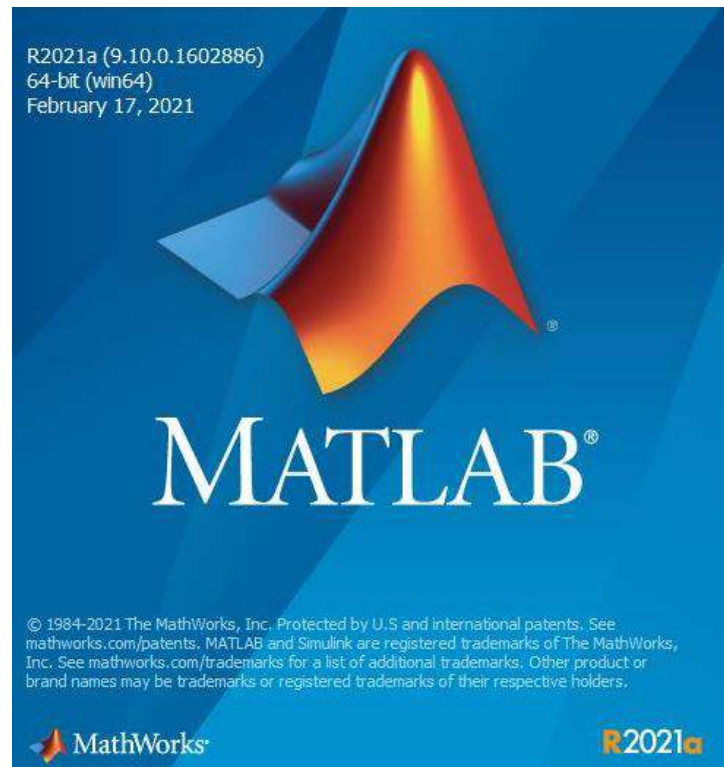


Fig 7: MATLAB LOADING INTERFACE

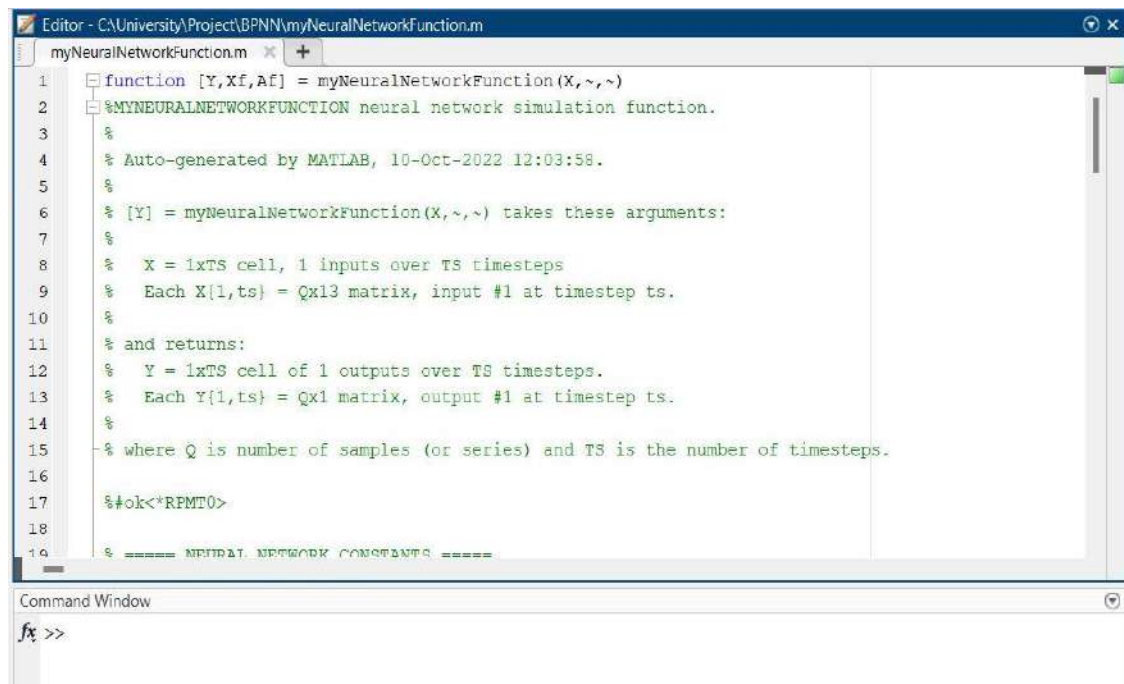


Fig 8: Interface

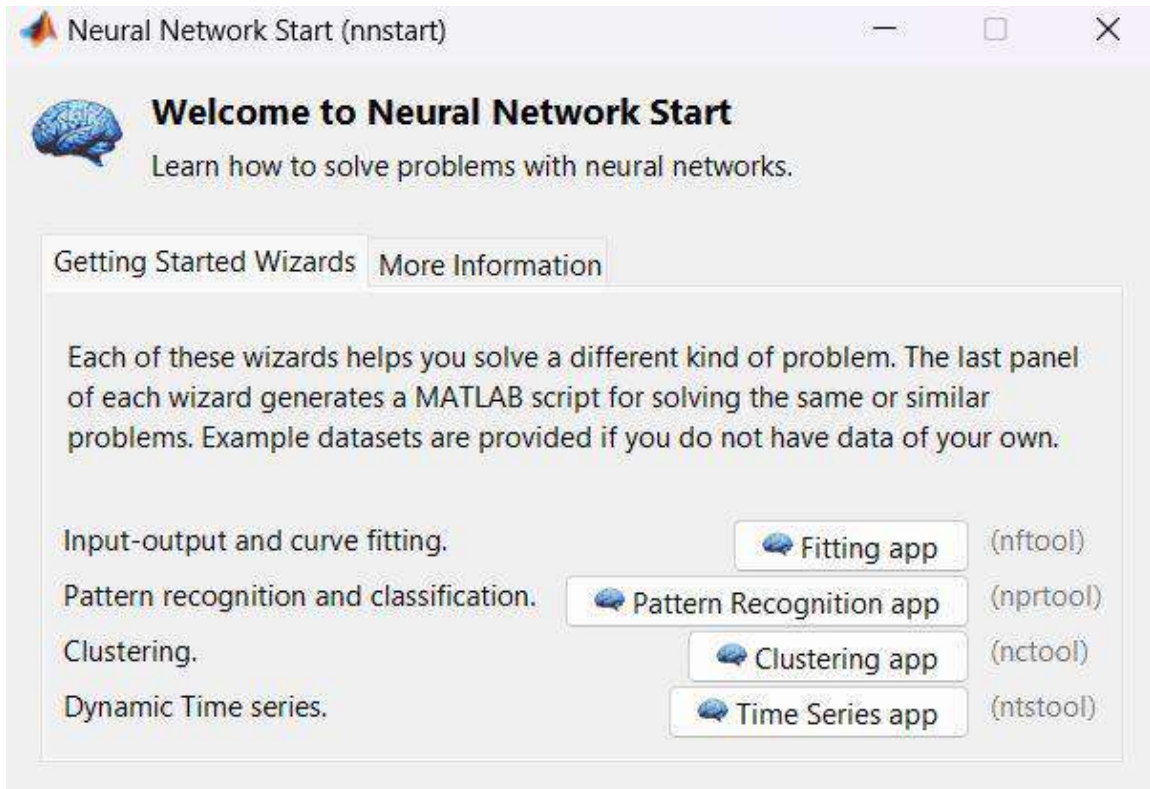


Fig 9: First Step after Interface

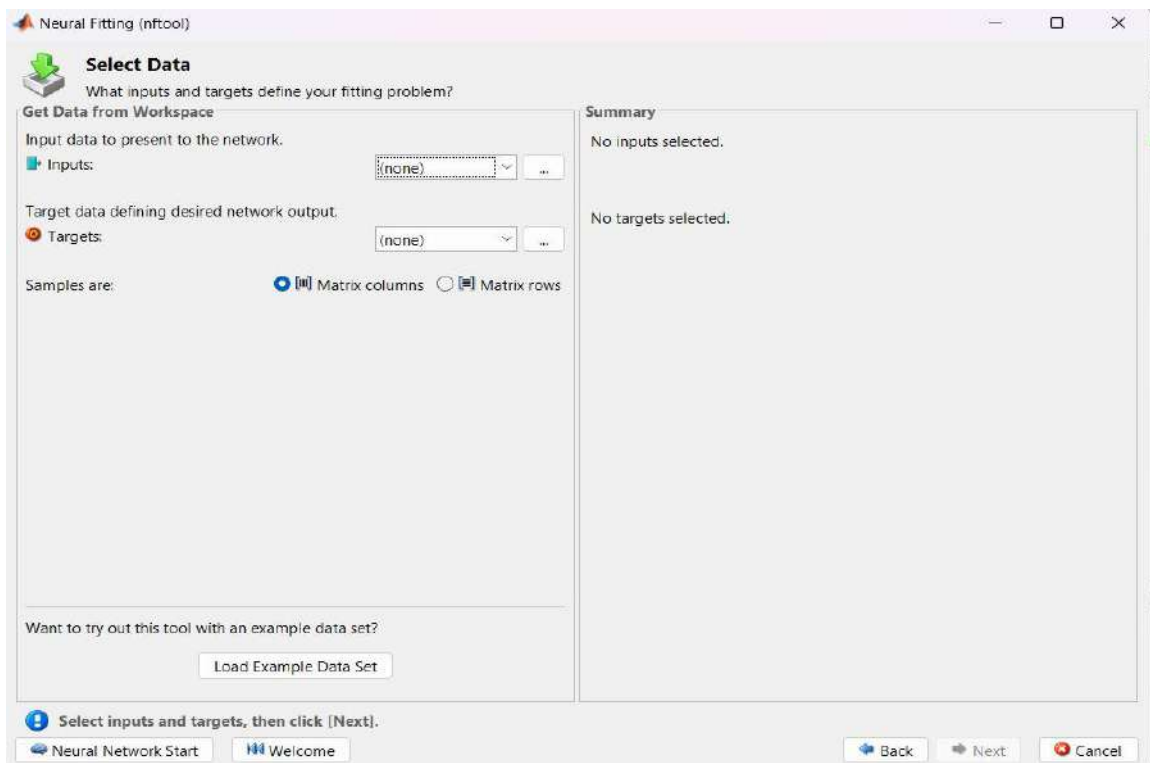


Fig 10: Selection of Inputs & Targets

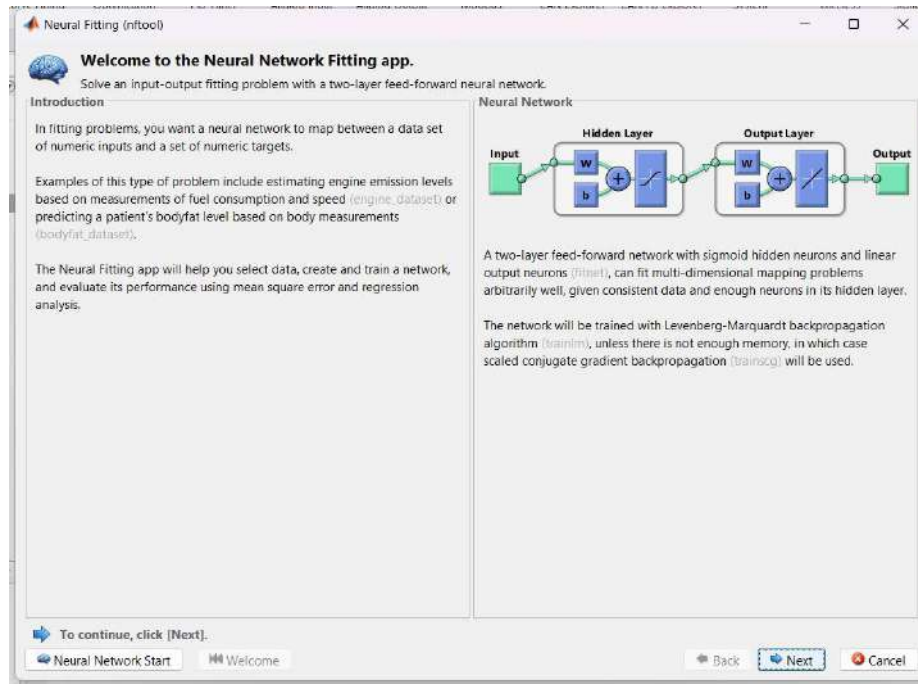


Fig 11: Neural Network Fitting Interface

### 3.2.3.2. VS CODE

Visual Studio Code, often called VS Code, is a code editor created by Microsoft using the Electron Framework. It is available on Windows, Linux, and macOS. It provides various functionalities such as debugging assistance, syntax highlighting, smart code suggestions, code shortcuts, code restructuring tools, and integrated Git support. Visual Studio Code (VS Code) is a versatile and widely-used code editor, especially favored by Python developers. It offers a powerful development environment that streamlines Python programming tasks. The editor's clean and user-friendly interface allows easy navigation and efficient project management. One of its standout features is the integrated terminal, which allows developers to run Python scripts and execute commands without leaving the editor. The code IntelliSense feature provides intelligent code completion, offering suggestions for Python functions, variable names, and available modules as you type, speeding up coding and reducing errors. Additionally, VS Code provides real-time linting and error checking, which helps identify syntax errors, style issues, and potential bugs in Python code. Its debugging support is another



significant advantage, enabling developers to set breakpoints, inspect variables, and step through code for effective troubleshooting. VS Code's extensibility is a major asset, boasting a vast collection of extensions, including many Python-specific ones, allowing developers to customize and enhance the editor's functionality according to their needs. Furthermore, with built-in Git integration, Python developers can easily manage version control and collaborate seamlessly. The editor also simplifies package management through an integrated terminal, facilitating the installation and management of Python packages. For those working with Jupyter Notebooks, VS Code provides native support, allowing Python developers to execute, edit, and visualize notebook files with ease.

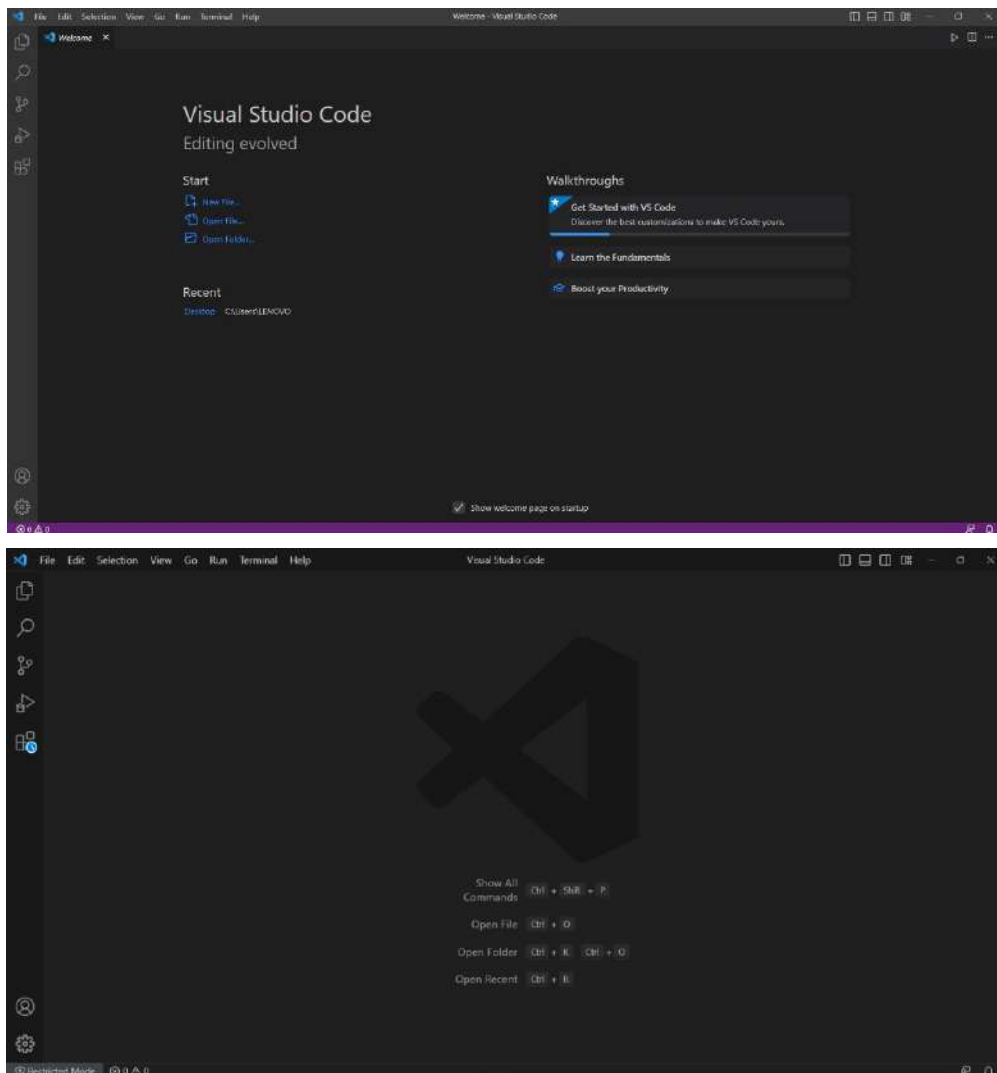


Fig 12: VS CODE Interface

### 3.3.DATA COLLECTION

The study collected data from 11 references (Aliabdo et al., 2016; Deb, 2013; Deb et al., 2014; Ghafoor et al., 2021a; Gunasekara et al., 2017, 2021; Joseph & Mathew, 2012; Kusbiantoro et al., 2012; Nath & Sarker, 2015; Sarker et al., 2013; Topark-Ngarm et al., 2015) with a total of 149 different mix proportions which consider the influence of Fly Ash, Coarse and Fine Aggregates, Alkaline Activators (NaOH & Na<sub>2</sub>SiO<sub>3</sub>), and Water is shown in Table II.

Table II: Proportion of geopolymer concrete using Fly Ash

FA (kg/m <sup>3</sup> )	Coarse aggregate (kg/m <sup>3</sup> )	Fine aggregate (kg/m <sup>3</sup> )	NaOH solution (kg/m <sup>3</sup> )	NaOH (M)	Na <sub>2</sub> SiO <sub>3</sub> solution (kg/m <sup>3</sup> )	Na <sub>2</sub> SiO <sub>3</sub> / NaOH	AA/ FA	Water (kg/m <sup>3</sup> )
400	1222	658	40	14	100	2.5	0.35	76.1

The molarity of the Sodium Hydroxide (NaOH) solution used was considered as one of the input parameters, with molarities ranging from 8 to 20. The mass of water was calculated for each mix proportion and added uniformly. The contents of Fly Ash were also considered as input parameters, specifically, the SiO<sub>2</sub> and Al<sub>2</sub>O<sub>3</sub> content. The AA/FA ratio and molarity of the alkaline solution were also used as input parameters. Coarse and fine aggregates were also taken as input parameters, with the raw material of each mix proportion and its size range mentioned in Table III. However, the fineness modulus of the fine aggregate and size range were not used due to difficulty in quantification. The temperature, which plays an important role in hydration, was also considered. Out of 149 mixes, 70 had high-temperature pre-curing (up to 100°C) and the pre-curing duration varied for each mix. The remaining 79 mixes did not have any pre-curing. A total of 13 inputs were used in the analysis, as listed in Table IV. The impact of each input parameter on the output was considered, and every single input from Table V was used to determine the applicability of the prediction model.

Table III: Particle size characteristics of aggregates

Reference	Coarse aggregate		Fineness modulus		
	Raw material	Size range (mm)	Raw material	Size range (mm)	Fineness modulus
(Gunasekara et al., 2017)	Crushed basalt	7.0-10.0	River sand		3
(Deb, 2013)	Crushed granite	7.0-20.0	Natural river sand	-	2.67
(Ghafoor et al., 2021a)	-	9.5-12.5	River sand	-	2.35
(Nath & Sarker, 2015)	Crushed granite	7.0-20.0	Natural sand	-	2.64
(Sarker et al., 2013)	Crushed stone	7.0-10.0	River sand	-	-
(Gunasekara et al., 2021)	Crushed granite	10	River sand		2.8
(Aliabdo et al., 2016)	Crushed bluestone gravel	7	Graded sand	<0.4	-
(Deb et al., 2014)	Crushed basalt aggregate	7.0-10.0	Uncrushed river sand	-	3
(Topark-Ngarm et al., 2015)	Limestone	20	River sand	-	2.9
(Joseph & Mathew, 2012)	Crushed granite rock	20	Natural river sand	-	2.64

Table IV: Composition of FA

Reference		SiO <sub>2</sub>	Al <sub>2</sub> O <sub>3</sub>	CaO	SO <sub>3</sub>	Fe <sub>2</sub> O <sub>3</sub>	MgO	LOI
(Gunasekara et al., 2017)	Pt. Augusta	49.37	31.25	4.8	0.24	4.47	1.28	0.51
	Collie	53.82	29.95	1.03	0.34	9.24	0.58	0.63
	Tarong	75.66	19	0.3	0.03	1.38	0	1.16
(Deb, 2013)		53.71	27.2	1.9	0.3	11.17	-	0.68
(Ghafoor et al., 2021a)		71.5	9.2	6.72	2.4	2.37	0.6	3.67
(Nath & Sarker, 2015)		53.71	27.2	1.9	0.3	11.17	-	0.68
(Kusbiantoro et al., 2012)		51.7	29.1	8.84	1.5	4.76	-	-
(Sarker et al., 2013)		50.5	26.57	2.13	0.41	13.77	1.54	0.6
(Gunasekara et al., 2021)		38.7	20.8	26.6	2.1	5.3	1.5	0.1
(Aliabdo et al., 2016)		49	31	5	-	3	3	-
(Deb et al., 2014)	Type-I	47.87	28	3.81	0.27	14.09	0.93	0.43
	Type-II	49.37	31.25	4.8	0.24	4.47	1.28	0.51
	Type-III	53.82	29.95	1.03	0.34	9.24	0.58	0.63
(Topark-Ngarm et al., 2015)		45.23	19.95	15.51	-	13.15	-	-
(Joseph & Mathew, 2012)		59.7	28.36	2.1	0.4	4.57	0.83	1.06
LOI: Loss of ignition								

Table V: Statistics of input/output parameters

Notation	Input parameter	Mean	Standard deviation	Range/categories		
				Minimum	Maximum	Range
X <sub>1</sub>	FA (kg/m <sup>3</sup> )	443.99	92.70	254.50	600	345.50
X <sub>2</sub>	SiO <sub>2</sub> (%)	50.99	14.57	36.20	75.66	39.46
X <sub>3</sub>	Al <sub>2</sub> O <sub>3</sub> (%)	18.23	6.60	9.20	31.25	22.05
X <sub>4</sub>	Coarse aggregate (kg/m <sup>3</sup> )	1105.97	231.54	554	1684	1130
X <sub>5</sub>	Fine aggregate (kg/m <sup>3</sup> )	594.33	45.56	500	706	206
X <sub>6</sub>	NaOH (kg/m <sup>3</sup> )	74.89	29.43	11.78	198	186.22
X <sub>7</sub>	NaOH (M)	11.88	3.01	8	20	12
X <sub>8</sub>	Na <sub>2</sub> SiO <sub>3</sub> (kg/m <sup>3</sup> )	165.18	67.31	29.51	342	312.46
X <sub>9</sub>	Na <sub>2</sub> SiO <sub>3</sub> /NaOH	2.29	0.78	1	8.77	7.77
X <sub>10</sub>	AA/FA	0.53	0.13	0.09	0.92	0.83
X <sub>11</sub>	Water (kg/m <sup>3</sup> )	143.24	44.41	37.45	206.78	169.33
X <sub>12</sub>	Temperature (°C)	29.02	32.16	0	100	100
X <sub>13</sub>	Duration (h)	13.69	17.26	0	72	72

In the collected data from various studies, the mixing methods for water and alkaline activators differed, with some combining them before mixing with the other components and others calculating the water content separately. To standardize the water content across all mix proportions, the total amount of water was determined by adding the mass of water in the NaOH and Na<sub>2</sub>SiO<sub>3</sub> solutions and any additional water utilized. Additionally, the variations in the chemical composition of Fly Ash (FA) used in the different studies were also taken into account in the analysis.

The analysis of the collected data considered the role of Fly Ash (FA) in the mix and incorporated the SiO<sub>2</sub> and Al<sub>2</sub>O<sub>3</sub> contents in FA as input parameters. This was in line with previous findings that reported the influence of Si and Al contents in FA on the strength development of FA-based mixes. The compressive strength of fly ash-based geopolymer mixes is influenced by several factors including the type and concentration of alkali solutions used, the Si/Al ratio in the fly ash, the calcium content, the curing conditions (such as temperature and duration), and the presence of any additional

additives. A higher Si/Al ratio leads to the formation of more -Si-O-Si- bonds, resulting in a stronger, fully condensed structural matrix of the geopolymer. This is because the -Si-O-Si- bonds are stronger than both the -Si-O-Al- and -Al-O-Al- bonds (Zhuang et al., 2016). In the preparation of fly ash-based geopolymer, the use of Na<sub>2</sub>SiO<sub>3</sub> solution in conjunction with NaOH has been shown to enhance compressive strength. The utilization of Na<sub>2</sub>SiO<sub>3</sub>, due to its high viscosity, promotes the formation of geopolymer gels which results in a compact microstructure of the final fly ash-based geopolymer. Additionally, the method used for activating the fly ash-based geopolymer also affects its compressive strength (Criado et al., 2005). Moreover, the curing duration and temperature play a significant role in determining the compressive strength of fly ash-based geopolymer. Prolonged curing, lasting from 6 hours to 28 days, results in fly ash-based geopolymer with greater compressive strength.

Elevated curing temperatures facilitate the removal of excess water from the fresh geopolymer, leading to the collapse of capillary pores and a denser structure, thus resulting in higher compressive strength (Leung & Pheeraphan, 1995). The higher NaOH concentration results in finer pore structures due to the polycondensation reaction, which leads to reduced chloride penetration. Additionally, geopolymer activated with NaOH has a more stable cross-linked structure, providing improved resistance to sulphates (Zhuang et al., 2016).

In this study, a range of input parameters related to concrete mixtures in civil engineering applications was analysed using statistical analysis. The parameters, such as Fine Aggregate, SiO<sub>2</sub> content, Al<sub>2</sub>O<sub>3</sub> content, Coarse Aggregate, Fine Aggregate, NaOH, Na<sub>2</sub>SiO<sub>3</sub>, Na<sub>2</sub>SiO<sub>3</sub>/NaOH ratio, AA/FA ratio, water content, temperature, and duration, were examined for their mean, standard deviation, minimum, maximum, and range values. Table V shows range of input/output parameters.

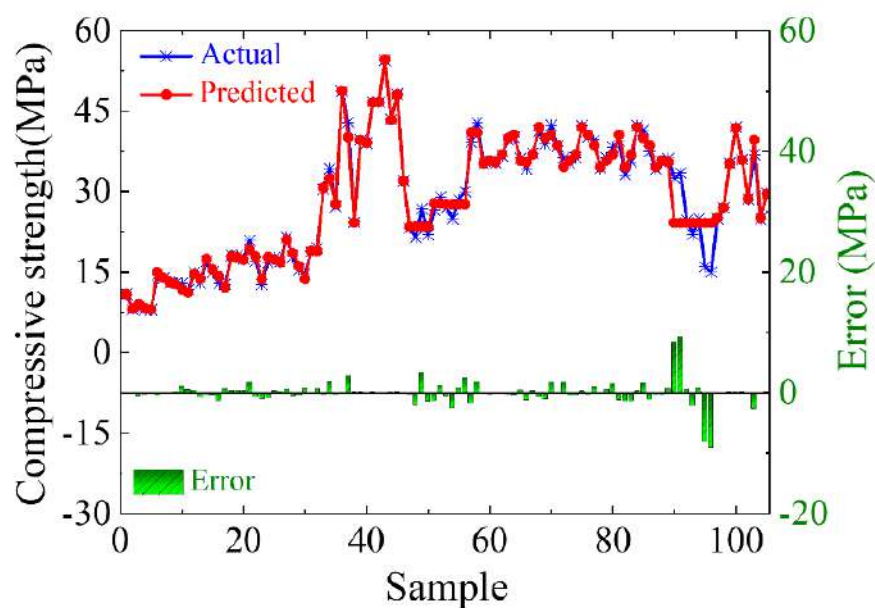
Table VI: Data set

Sr #	FA	SiO2	Al2O3	Coarse aggregate	Fine aggregate	NaOH	NaOH (M)	Na2SiO3	Na2SiO3/NaOH	AA/FA	Water	Temperature	Duration	fcu
-	kg/m3	kg/m3	kg/m3	kg/m3	kg/m3	kg/m3		kg/m3	-	-	kg/m3	°C	Days	Mpa
1	394	71.5	9.2	1294	554	63	8	94.6	1.5	0.4	117.4	0	0	10.9
2	394	71.5	9.2	1294	554	52.5	8	105.1	2	0.4	115.3	0	0	9.2
3	394	71.5	9.2	1294	554	45	8	112.6	2.5	0.4	113.8	0	0	8
4	368	71.5	9.2	1294	554	73.6	8	110.4	1.5	0.5	132.48	0	0	7.6
5	368	71.5	9.2	1294	554	61.3	8	122.6	2	0.5	129.97	0	0	8.5
6	368	71.5	9.2	1294	554	52.5	8	131.4	2.5	0.5	128.21	0	0	8.5
7	345	71.5	9.2	1294	554	82.8	8	124.2	1.5	0.6	145.59	0	0	7.6
8	345	71.5	9.2	1294	554	69	8	138	2	0.6	142.83	0	0	8
9	345	71.5	9.2	1294	554	59.1	8	147.8	2.5	0.6	140.8	0	0	8
10	394	71.5	9.2	1294	554	63	10	94.6	1.5	0.4	114.25	0	0	14.4
11	394	71.5	9.2	1294	554	52.5	10	105.1	2	0.4	112.68	0	0	13.9
12	394	71.5	9.2	1294	554	45	10	112.6	2.5	0.4	111.55	0	0	13.1
13	368	71.5	9.2	1294	554	73.6	10	110.4	1.5	0.5	128.8	0	0	14.2
14	368	71.5	9.2	1294	554	61.3	10	122.6	2	0.5	126.9	0	0	12.9
15	368	71.5	9.2	1294	554	52.5	10	131.4	2.5	0.5	125.58	0	0	12.9
16	345	71.5	9.2	1294	554	82.8	10	124.2	1.5	0.6	141.45	0	0	13
17	345	71.5	9.2	1294	554	69	10	138	2	0.6	139.38	0	0	11.7
18	345	71.5	9.2	1294	554	59.1	10	147.8	2.5	0.6	137.84	0	0	11.8
19	394	71.5	9.2	1294	554	63	12	94.6	1.5	0.4	111.1	0	0	15
20	394	71.5	9.2	1294	554	52.5	12	105.1	2	0.4	110.05	0	0	14.5
21	394	71.5	9.2	1294	554	45	12	112.6	2.5	0.4	109.3	0	0	13
22	368	71.5	9.2	1294	554	73.6	12	110.4	1.5	0.5	125.12	0	0	17
23	368	71.5	9.2	1294	554	61.3	12	122.6	2	0.5	123.84	0	0	15
24	368	71.5	9.2	1294	554	52.5	12	131.4	2.5	0.5	122.96	0	0	12.9
25	345	71.5	9.2	1294	554	82.8	12	124.2	1.5	0.6	137.31	0	0	12.5
26	345	71.5	9.2	1294	554	69	12	138	2	0.6	135.93	0	0	13
27	345	71.5	9.2	1294	554	59.1	12	147.8	2.5	0.6	134.89	0	0	12.7
28	394	71.5	9.2	1294	554	63	14	94.6	1.5	0.4	108.58	0	0	18.1
29	394	71.5	9.2	1294	554	52.5	14	105.1	2	0.4	107.95	0	0	18
30	394	71.5	9.2	1294	554	45	14	112.6	2.5	0.4	107.5	0	0	17.5
31	368	71.5	9.2	1294	554	73.6	14	110.4	1.5	0.5	122.18	0	0	18.7
32	368	71.5	9.2	1294	554	61.3	14	122.6	2	0.5	121.38	0	0	20.9
33	368	71.5	9.2	1294	554	52.5	14	131.4	2.5	0.5	120.86	0	0	17.1
34	345	71.5	9.2	1294	554	82.8	14	124.2	1.5	0.6	134	0	0	21.5
35	345	71.5	9.2	1294	554	69	14	138	2	0.6	133.17	0	0	16
36	345	71.5	9.2	1294	554	59.1	14	147.8	2.5	0.6	132.52	0	0	12.7
37	394	71.5	9.2	1294	554	63	16	94.6	1.5	0.4	106.06	0	0	16.8
38	394	71.5	9.2	1294	554	52.5	16	105.1	2	0.4	105.85	0	0	17.6
39	394	71.5	9.2	1294	554	45	16	112.6	2.5	0.4	105.7	0	0	17
40	368	71.5	9.2	1294	554	73.6	16	110.4	1.5	0.5	119.23	0	0	21.5
41	368	71.5	9.2	1294	554	61.3	16	122.6	2	0.5	118.93	0	0	17.9
42	368	71.5	9.2	1294	554	52.5	16	131.4	2.5	0.5	118.76	0	0	15.8
43	345	71.5	9.2	1294	554	82.8	16	124.2	1.5	0.6	130.69	0	0	17
44	345	71.5	9.2	1294	554	69	16	138	2	0.6	130.41	0	0	15.5
45	345	71.5	9.2	1294	554	59.1	16	147.8	2.5	0.6	130.16	0	0	14.6
46	400	49	31	1280	547	19.8	14	49.6	2.5	0.17	130.6	0	0	18.8
47	400	49	31	1280	547	19.8	14	49.6	2.5	0.17	130.6	0	0	19.5
48	475	49	31	1253	539	11.781	14	29.512	2.5	0.09	77.71	0	0	30.4
49	475	49	31	1248	535	13.86	14	34.72	2.5	0.1	91.42	0	0	34.3
50	475	49	31	1235	529	16.434	14	41.168	2.5	0.12	108.4	0	0	27.2
51	416	47.87	28	927	699	65	15	292	4.5	0.86	197.23	80	24	48.7
52	416	49.37	31.25	927	699	65	15	292	4.5	0.86	197.23	80	24	42.9
53	420	53.82	29.95	936	706	92	15	241	2.6	0.79	186.52	80	24	24.3
54	414	45.23	19.95	1091	588	104	10	104	1	0.5	116.27	0	0	39.67
55	414	45.23	19.95	1091	588	104	15	104	1	0.5	95.47	0	0	45.34
56	414	45.23	19.95	1091	588	104	20	104	1	0.5	74.67	0	0	37.64
57	414	45.23	19.95	1091	588	69	10	138	2	0.5	112.88	0	0	33.8
58	414	45.23	19.95	1091	588	69	15	138	2	0.5	99.08	0	0	39.02
59	414	45.23	19.95	1091	588	69	20	138	2	0.5	85.28	0	0	46.69
60	414	45.23	19.95	1091	588	104	10	104	1	0.5	116.27	60	24	46.67
61	414	45.23	19.95	1091	588	104	15	104	1	0.5	95.47	60	24	54.4
62	414	45.23	19.95	1091	588	104	20	104	1	0.5	74.67	60	24	43.42
63	414	45.23	19.95	1091	588	69	10	138	2	0.5	112.88	60	24	40.09
64	414	45.23	19.95	1091	588	69	15	138	2	0.5	99.08	60	24	48.18
65	414	45.23	19.95	1091	588	69	20	138	2	0.5	85.28	60	24	49.5
66	408	50.5	26.57	554	647	62	14	93	1.5	0.379902	98.612	60	24	32
67	300	38.7	20.8	1684	681	51.4	14	129	2.50973	0.601333	94.727	60	24	25.8
68	300	38.7	20.8	1684	681	51.4	14	129	2.50973	0.601333	94.727	60	24	23.2
69	300	38.7	20.8	1684	681	51.4	14	129	2.50973	0.601333	94.727	60	24	21.5
70	300	38.7	20.8	1684	681	51.4	14	129	2.50973	0.601333	94.727	60	24	26.8
71	300	38.7	20.8	1684	681	51.4	14	129	2.50973	0.601333	94.727	60	24	20.5
72	300	38.7	20.8	1684	681	51.4	14	129	2.50973	0.601333	94.727	60	24	22
73	600	38.7	20.8	1087	572	89.1	14	223	2.50281	0.520167	163.861	60	24	26.5
74	600	38.7	20.8	1087	572	89.1	14	223	2.50281	0.520167	163.861	60	24	29
75	600	38.7	20.8	1087	572	89.1	8	223	2.50281	0.520167	185.245	60	24	27

76	600	38.7	20.8	1087	572	89.1	8	223	2.50281	0.520167	185.245	60	24	25
77	600	38.7	20.8	1087	572	89.1	8	223	2.50281	0.520167	185.245	60	24	22.5
78	600	38.7	20.8	1087	572	89.1	8	223	2.50281	0.520167	185.245	60	24	28.5
79	600	38.7	20.8	1087	572	89.1	8	223	2.50281	0.520167	185.245	60	24	22
80	600	38.7	20.8	1087	572	89.1	8	223	2.50281	0.520167	185.245	60	24	30
81	494	38.7	20.8	858	691	198	14	198	1	0.801619	196.02	60	72	39.2
82	494	38.7	20.8	858	691	198	14	198	1	0.801619	196.02	60	72	36.5
83	494	38.7	20.8	858	691	198	14	198	1	0.801619	196.02	60	72	42.8
84	450	36.2	19.9	1150	500	108	12	162	1.5	0.6	149.148	60	48	35.2
85	450	36.2	19.9	1036	500	108	12	162	1.5	0.6	149.148	60	48	32.9
86	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	60	24	33.2
87	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	0	0	35.6
88	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	60	24	35.4
89	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	0	0	36.7
90	550	38.7	20.8	838	600	95	12	239	2.51579	0.607273	188.02	60	24	42.4
91	550	38.7	20.8	838	600	95	12	239	2.51579	0.607273	188.02	0	0	39.7
92	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	60	24	40.1
93	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	0	0	38.7
94	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	60	24	34.7
95	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	0	0	36.2
96	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	60	24	34.3
97	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	0	0	37.1
98	550	38.7	20.8	838	600	95	12	239	2.51579	0.607273	188.02	60	24	41.3
99	550	38.7	20.8	838	600	95	12	239	2.51579	0.607273	188.02	0	0	38.9
100	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	60	24	42.3
101	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	0	0	38.5
102	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	60	24	36.3
103	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	0	0	35.3
104	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	60	24	36.1
105	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	0	0	36.3
106	550	38.7	20.8	838	600	95	12	239	2.51579	0.607273	188.02	60	24	42.2
107	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	60	24	40.2
108	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	0	0	39.6
109	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	60	24	34.4
110	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	0	0	36.3
111	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	60	24	35.4
112	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	0	0	38.3
113	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	60	24	39.4
114	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	0	0	38.3
115	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	60	24	33.1
116	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	0	0	33.5
117	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	0	24	35.1
118	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	0	0	35.5
119	550	38.7	20.8	838	600	95	12	239	2.51579	0.607273	188.02	60	24	42.2
120	550	38.7	20.8	838	600	95	12	239	2.51579	0.607273	188.02	0	0	41.5
121	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	60	24	40.2
122	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	0	0	37.5
123	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	60	24	34.2
124	550	38.7	20.8	838	600	95	8	239	2.51579	0.607273	203.22	0	0	35.5
125	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	60	24	36.2
126	550	38.7	20.8	838	600	95	10	239	2.51579	0.607273	195.62	0	0	37.5
127	550	38.7	20.8	838	600	95	12	239	2.51579	0.607273	188.02	60	24	41.4
128	550	38.7	20.8	838	600	95	12	239	2.51579	0.607273	188.02	0	0	40.5
129	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	60	24	40.8
130	550	38.7	20.8	838	600	95	14	239	2.51579	0.607273	180.42	0	0	38.4
131	350	38.7	20.8	1250	650	41	8	103	2.5122	0.411429	85.045	60	24	32.5
132	350	38.7	20.8	1250	650	41	8	103	2.5122	0.411429	85.045	60	24	33.5
133	350	38.7	20.8	1250	650	41	8	103	2.5122	0.411429	85.045	60	24	31
134	350	38.7	20.8	1250	650	41	8	103	2.5122	0.411429	85.045	60	24	24.7
135	350	38.7	20.8	1250	650	41	8	103	2.5122	0.411429	85.045	60	24	22
136	350	38.7	20.8	1250	650	41	8	103	2.5122	0.411429	85.045	60	24	25
137	350	38.7	20.8	1250	650	41	8	103	2.5122	0.411429	85.045	60	24	23.5
138	350	38.7	20.8	1250	650	41	8	103	2.5122	0.411429	85.045	60	24	16
139	350	38.7	20.8	1250	650	41	8	103	2.5122	0.411429	85.045	60	24	15
140	400	53.71	27.2	1222	658	40	14	100	2.5	0.35	76.1	21.5	72	25
141	400	53.71	27.2	1222	658	56	14	84	1.5	0.35	74.76	21.5	72	27
142	365.2	59.7	28.36	1118	602	34.3	10	73	2.12828	0.293812	56.35	100	24	35.3
143	254.5	59.7	28.36	1290	694.7	22.8	10	48.5	2.12719	0.280157	37.445	100	24	36.8
144	309.9	59.7	28.36	1204	648.4	27.7	10	59	2.12996	0.279768	45.53	100	24	42
145	408	51.7	29.1	1294	554	41	14	103	2.5122	0.352941	121.0086	60	24	36
146	428.6	53.71	27.2	1177	623	68.6	14	102.9	1.5	0.40014	98.7154	21.5	72	28.6
147	416	49.37	31.25	927	699	65	15	292	4.49231	0.858173	197.228	80	24	36.9
148	420	53.82	29.95	936	706	92	15	241	2.61957	0.792857	171.919	80	24	24.9
149	412	75.66	19	918	693	39	15	342	8.76923	0.924757	206.778	80	24	29.6

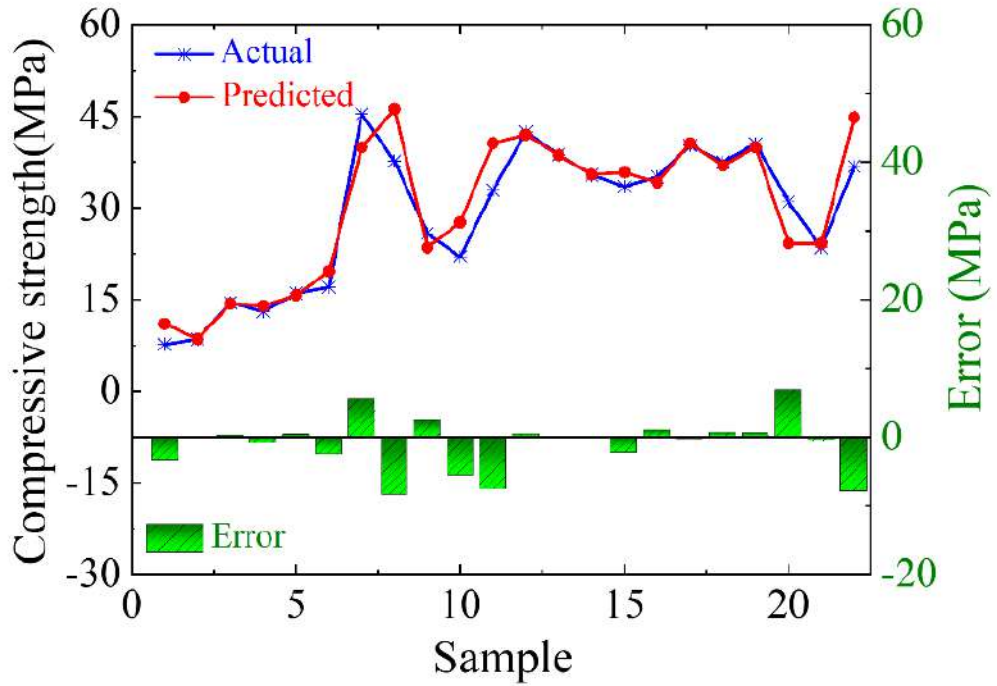
#### 4. RESULTS AND DISCUSSION

In this chapter the outcomes and results of the whole research are discussed such as machine learning predictions of 28 days compressive strength of fly ash-based geopolymer concrete mixtures using three algorithms BPNN, RFR, and KNN as shown in Fig.13-15. Red points indicate predicted values, whereas blue points reflect measured values. The gray histograms beneath each graphic show the difference between predicted and measured values. The models agree well with the training data, and their predictions for 28-day compressive strength are correct, with errors of less than 10 MPa. Overall, all three models worked admirably, precisely matching the measured compressive strength. Among them, BPNN demonstrated the highest level of accuracy.

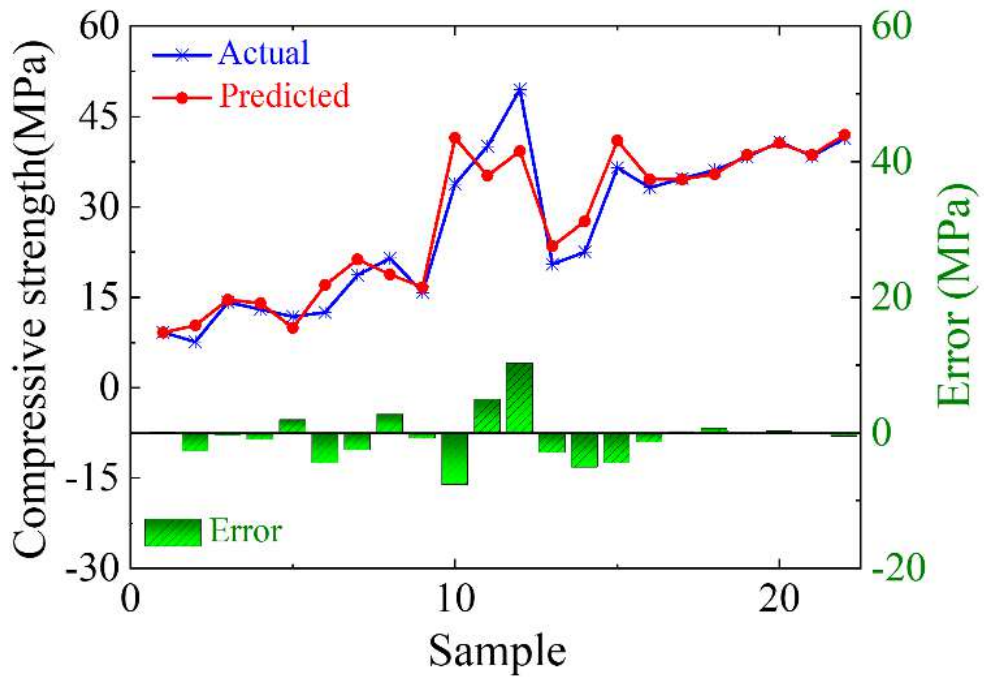


(a) Training set



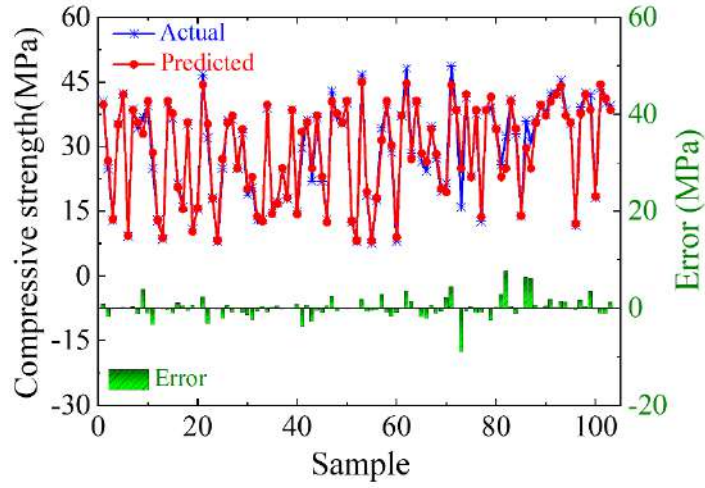


(b) Validation set

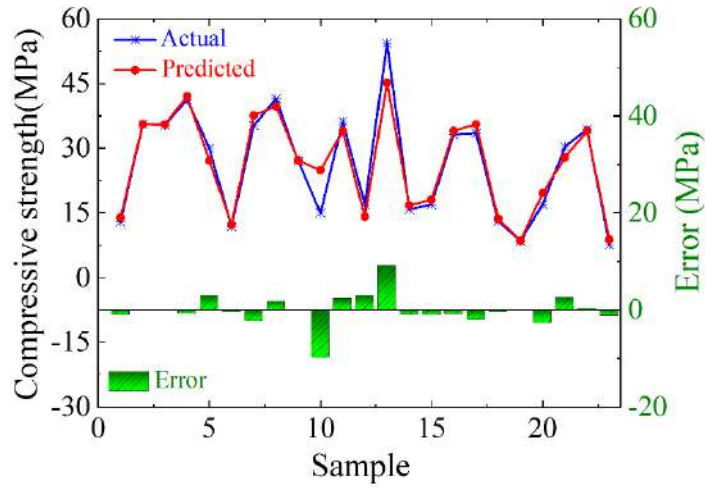


(c) Testing set

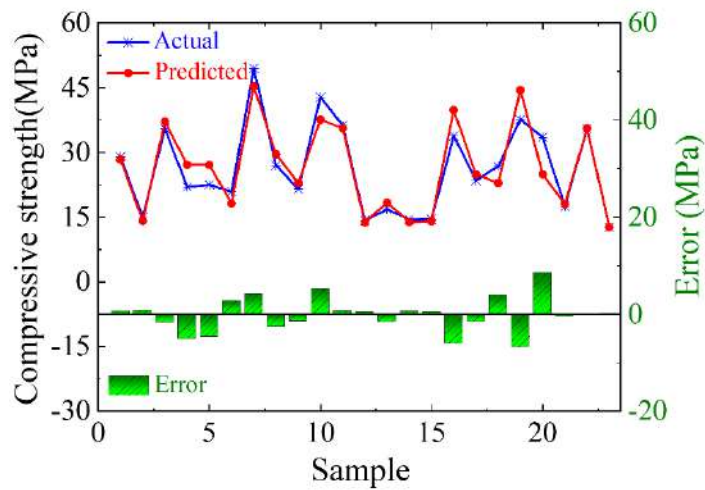
Fig 13: Actual and predicted compressive strength by BPNN



(a) Training set

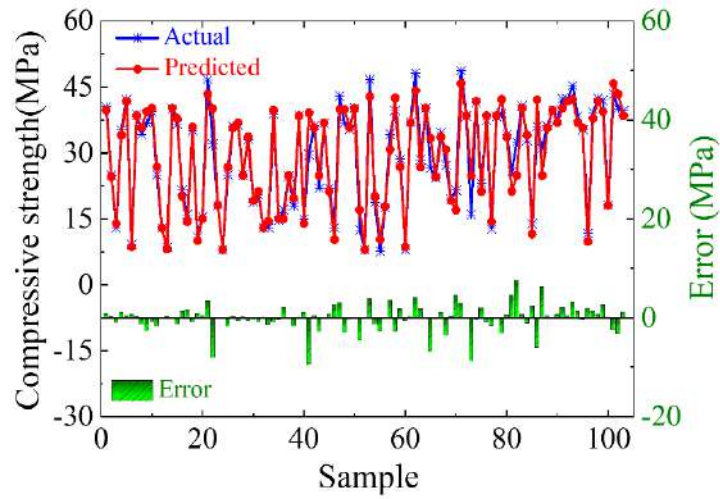


(b) Validation set

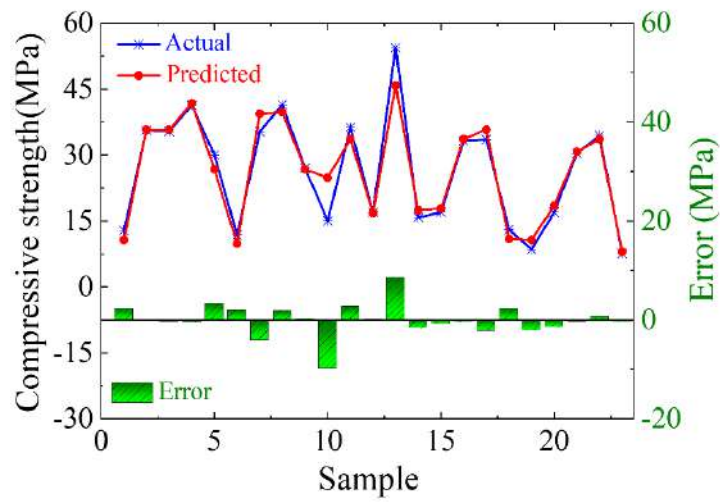


(c) Testing set

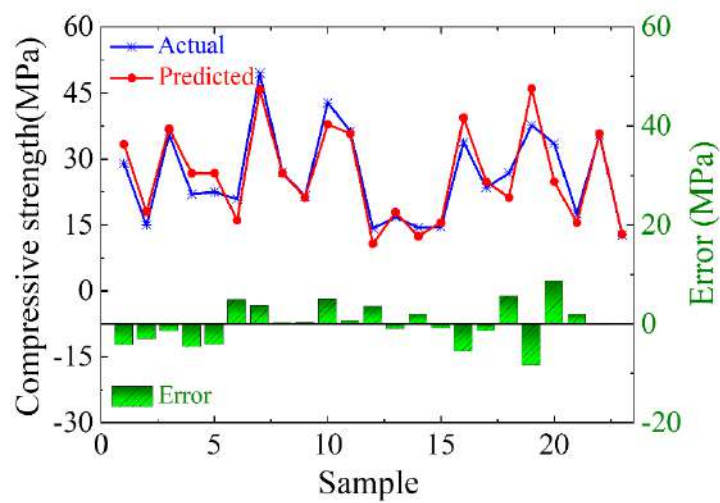
Fig 14: Actual and predicted compressive strength by RFR



(a) Training set



(b) Validation set



(c) Testing set

Fig 15: Actual and predicted compressive strength by KNN

#### 4.1.COMPARISON OF NEURAL NETWORK MODELS

Table VII present the evaluation results for three models: BPNN, RFR, and KNN, indicating similar performance trends. On average, T BPNN exhibited the highest accuracy, with an impressive  $R^2$  score of above 94% for the entire dataset. All three algorithms showed remarkable accuracy, with results in training, validation, and testing sets closely aligned. However, when comparing  $R^2$  values, BPNN had slightly lower validation  $R^2$  value of 0.897 compared to RFR  $R^2$  value of 0.932 and KNN 0.933. Yet, BPNN performed better in testing  $R^2$  value of 0.9174 compared to KNN 0.852 and RFR 0.878. Overall, the models showed an efficiency of over 90%.

Table VII: Accuracy comparison of three machine learning algorithms

	BPNN				RFR				KNN			
	Training set	Validation set	Testing set	All	Trainin g set	Validation set	Testing set	All	Trainin g set	Validation set	Testing set	All
$R^2$	0.970	0.897	0.917	0.948	0.971	0.932	0.878	0.927	0.947	0.933	0.852	0.911
$MSE$	3.90	15.27	13.36	10.84	3.87	10.45	12.31	8.88	6.97	10.23	14.92	10.71
$RMSE$	1.97	3.91	3.65	3.29	1.97	3.20	3.47	2.95	2.65	3.22	3.97	3.33
$MAE$	1.01	2.60	2.65	2.09	1.24	1.95	2.61	1.93	1.72	2.05	2.96	2.25

The superiority of BPNN over KNN and RFR, despite lower validation results, can be attributed to BPNN's capability to capture complex patterns in the data better than other models. Neural networks possess unique characteristics such as fault tolerance, non-linearity, self-learning, self-organization, and self-adaptation, which give BPNN an advantage in handling complex relationships.

BPNN is an optimization algorithm specifically designed to minimize the sum of squared errors, making it well-suited for modelling complex, nonlinear relationships between input features and output values. This flexibility allows it to capture the intricacies of the data better than other models like RFR, which rely on predefined

decision rules. As a result, BPNN can often find a better fit to the data and deliver improved predictive accuracy.

Fig.16 illustrates the results of predicted and experimental compressive strength obtained from three models, BPNN, RFR, and KNN. The data points generated by these algorithms generally clustered around the centre line and fell within an error range of 10%. Notably, most of the errors between actual and predicted values were below 10% across all the models. The close alignment of data points around the centre line signifies a high level of accuracy in the predictions made by BPNN, RFR, and KNN for the compressive strength of the geopolymers concrete.

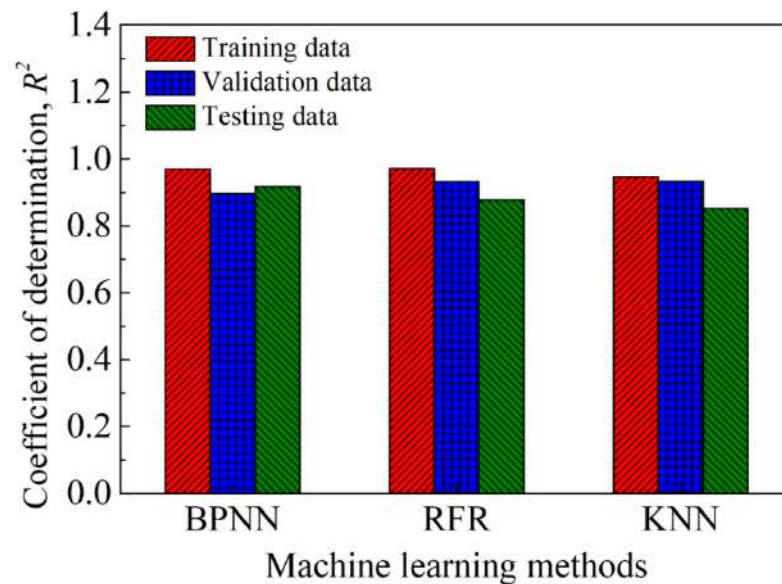
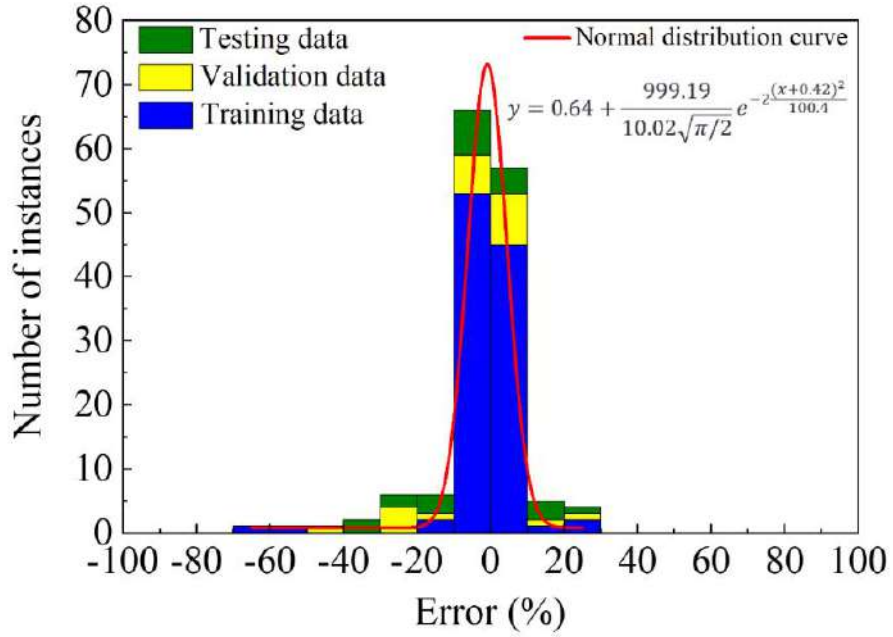


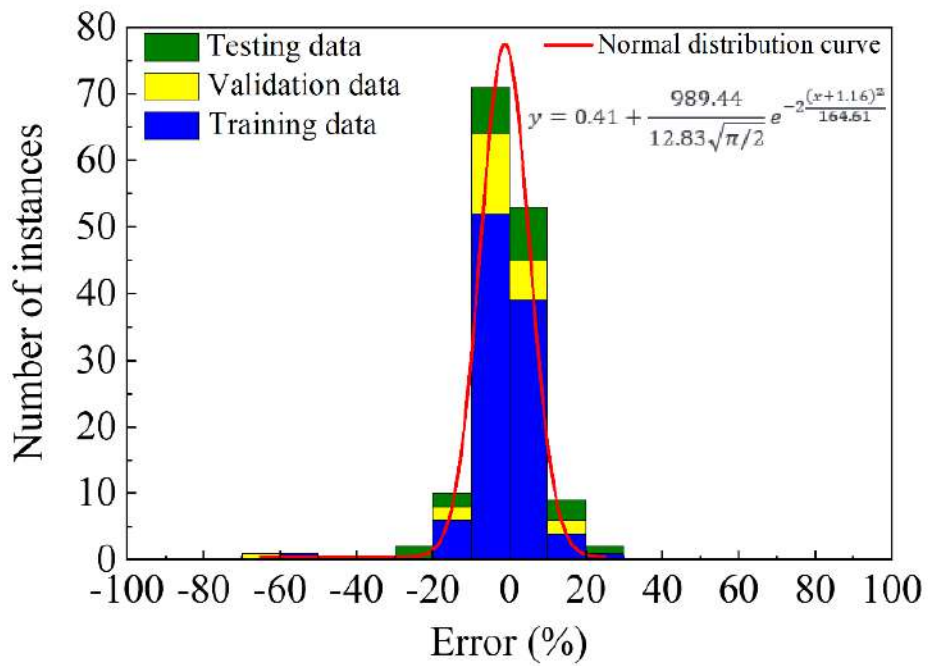
Fig 16: Comparison of coefficient of determination ( $R^2$ )

Fig.17 displays the error percentages and their frequency for different machine learning algorithms. To ensure precise measurements, we used error percentage instead of absolute error, and then fit the data to a normal distribution. This allowed us to eliminate the influence of sample size on deviation. The results showed that all three algorithms had an average error percentage close to zero. Among them, RFR had the steepest fitting curve, indicating a more concentrated distribution within the -20% to +20% error range.

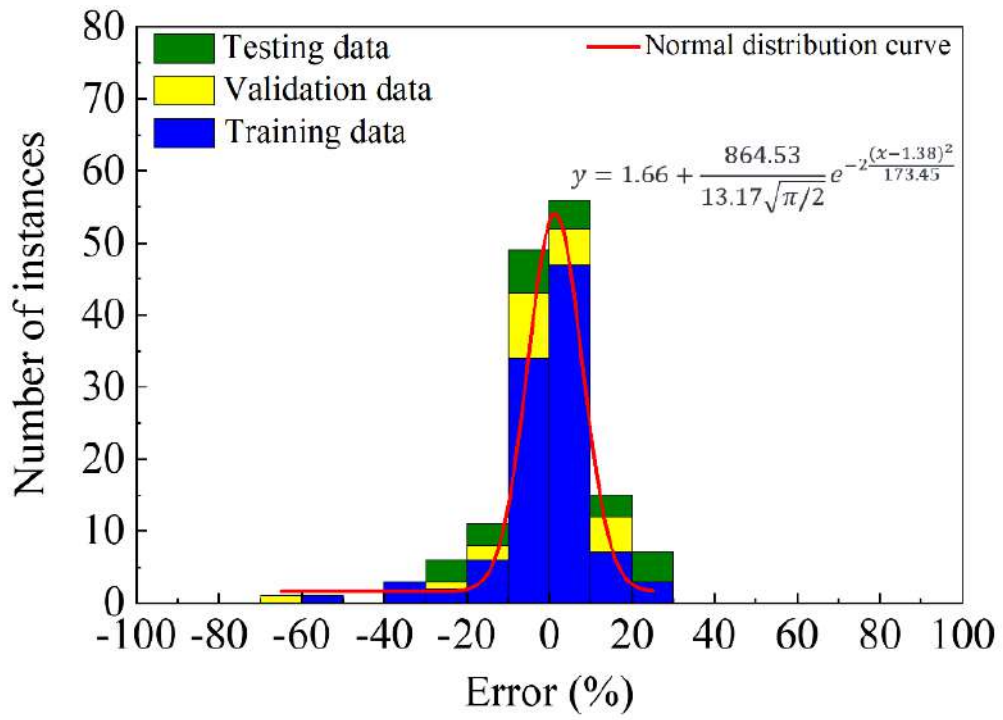
However, BPNN produced predictions that were closer to the actual results, even though it had fewer samples in the -20% to +20% error range than RFR. These findings reinforce our earlier conclusion that BPNN showed superior prediction accuracy as illustrated in Fig.18.



(a) BPNN

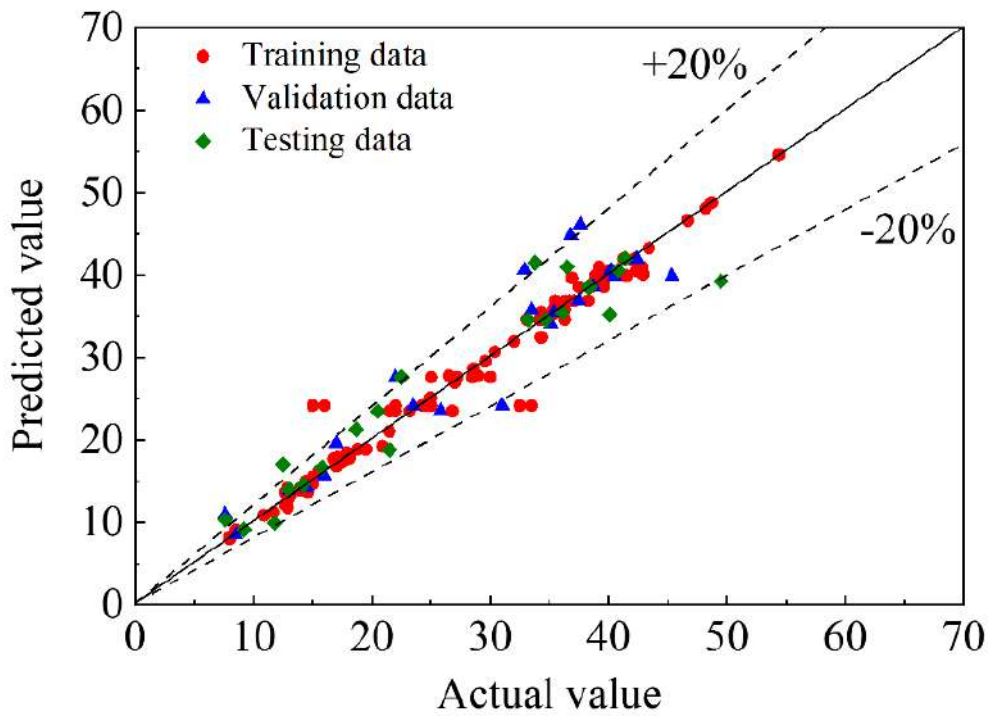


(b) RFR

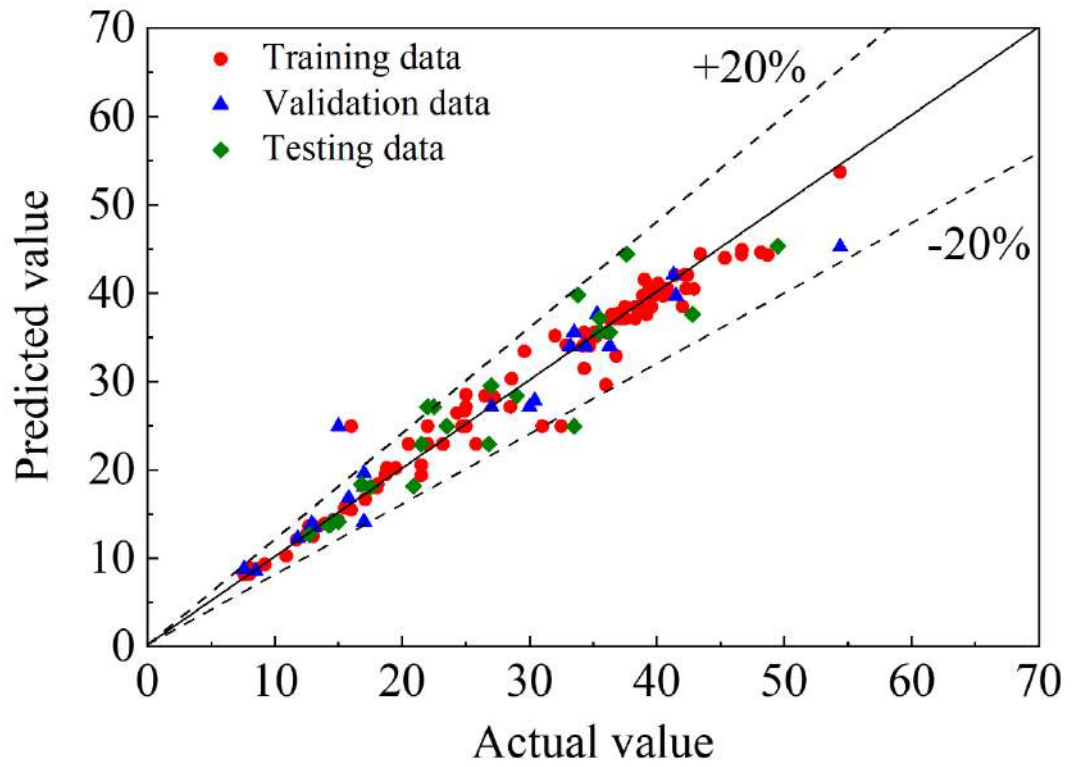


(c) KNN

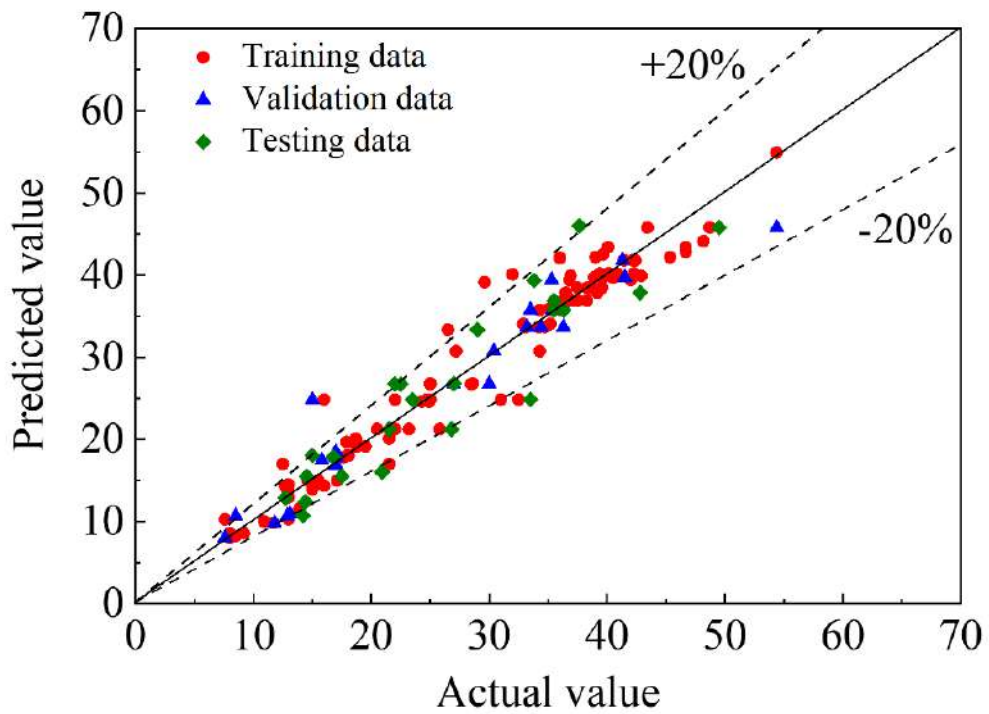
Fig 17: Normal distribution fitting and error percentage distribution of models



(a) BPNN



(b) RFR



(c) KNN

Fig 18: Comparison of actual and predicted values of compressive strength



#### 4.2.EFFECTS OF INPUT PARAMETERS

The results of our study on the impact of various mix design parameters on the compressive strength of FA-based geopolymer concrete are presented in Fig.19. The findings showed that there was no significant correlation between any of the variables and compressive strength. This result may have been due to the use of a large number of variables with widely varying ranges in our study. This reduction of the impact of any one factor on the outcomes is a common challenge in multi-variable regression analysis. However, by examining the correlation between a few independent variables and several dependent variables, some potential trends might still be found. The results showed that an ideal  $\text{SiO}_2/\text{Al}_2\text{O}_3$  ratio was revealed by the findings of the  $\text{SiO}_2$  and  $\text{Al}_2\text{O}_3$  contents, which improved the contribution of FA to compressive strength. This highlights the importance of considering the ratio of these two components in the mix design of FA-based geopolymer concrete. Further research on a smaller set of variables could lead to more definitive conclusions about the impact of specific parameters on compressive strength. The compressive strength of FA-based geopolymer concrete was found to be impacted by various factors such as the amount of NaOH and  $\text{Na}_2\text{SiO}_3$  as well as their molar concentrations. Results showed that an increase in the molar concentration of NaOH solution increased the compressive strength, particularly when the concentration exceeded 12 M. Conversely, a rise in the water content above 100  $\text{kg/m}^3$  led to a decrease in strength.

The proportions of coarse or fine particles, on the other hand, did not appear to have a significant effect on strength. This aligns with previous studies that have revealed that the aggregates' morphology, particle size distribution, and interface transition zone have a greater impact on strength growth compared to their contents. The effect of pre-curing

on compressive strength is detailed in Table VIII, which also shows the effect on the minimum, average, and maximum strength values across all data sets.

Table VIII: Impact of pre-curing on the compressive strength

Curing condition	Compressive strength (MPa)		
	Minimum	Maximum	Average
No pre-curing	7.60	46.69	23.63
Pre-curing (up to 100°C and 72 h)	15	54.40	33.55

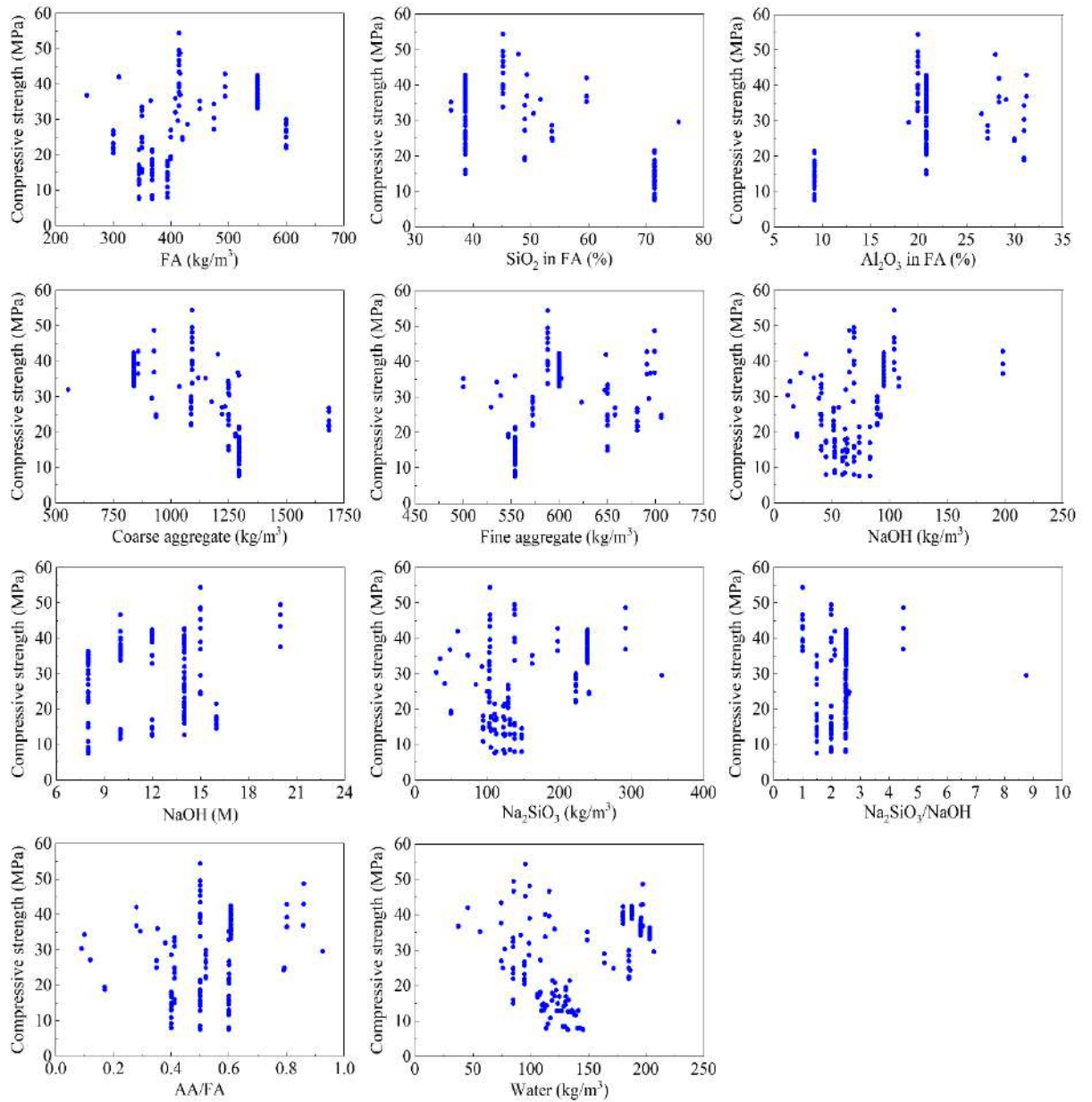


Fig 19: Correlation of different input parameters with Compressive strength

To comprehend the significance of each input parameter concerning prediction outcomes, evaluating the importance of individual parameters is crucial. This is often done through the utilization of permutation feature importance (PFI), a widely used metric for assessing the influence of various features on outcomes. PFI operates on the principle that when an input variable, let's call it  $X_i$ , significantly affects the outcome, shuffling  $X_i$  while maintaining the order of other variables will notably decrease predictive performance. This concept is expressed in Equation 13, which calculates the PFI value for a specific variable using mean absolute error (MAE) as the error measurement. Here,  $MAE_{perm}$  represents the mean absolute error after randomly rearranging  $X_i$ , while  $MAE_{orig}$  represents the original mean absolute error.

$$PFI = MAE_{perm} - MAE_{orig} \quad (13)$$

By applying this approach, a PFI value approaching zero indicates that modifying a particular feature has minimal impact on the output. Conversely, a substantial PFI value suggests that the feature significantly influences the output. In a recent study, this PFI method was employed to assess the impact of 13 input variables on the compressive strength of geopolymer concrete as shown in Table IX.

Table IX: PFI results for different input parameters

Notation	Input parameter	$MAE_{perm}$	$PFI$
X <sub>1</sub>	FA (kg/m <sup>3</sup> )	1.77	0.78
X <sub>2</sub>	SiO <sub>2</sub> (%)	3.24	2.25
X <sub>3</sub>	Al <sub>2</sub> O <sub>3</sub> (%)	2.32	1.33
X <sub>4</sub>	Coarse Aggregate (kg/m <sup>3</sup> )	6.75	5.76
X <sub>5</sub>	Fine Aggregate (kg/m <sup>3</sup> )	2.52	1.53
X <sub>6</sub>	NaOH (kg/m <sup>3</sup> )	1.80	0.81
X <sub>7</sub>	NaOH (M)	2.94	1.95
X <sub>8</sub>	Na <sub>2</sub> SiO <sub>3</sub> (kg/m <sup>3</sup> )	1.39	0.40
X <sub>9</sub>	Na <sub>2</sub> SiO <sub>3</sub> /NaOH	1.38	0.39
X <sub>10</sub>	AA/FA	1.24	0.25
X <sub>11</sub>	Water (kg/m <sup>3</sup> )	2.05	1.06
X <sub>12</sub>	Temperature (°C)	1.42	0.43
X <sub>13</sub>	Duration (h)	1.31	0.32

The study's findings highlighted that the concrete's compressive strength was notably affected by the quantity of coarse aggregate present in the mix. This is attributed to the substantial volume occupied by coarse aggregates, significantly influencing the overall packing density and void content. Similar observations regarding the influence of coarse aggregates on concrete's compressive strength have been reported in earlier studies (Olivia & Nikraz, 2012),(Malkawi, 2023). Alongside coarse aggregates, the SiO<sub>2</sub> percentage in the fly ash (FA) also demonstrated a high PFI value, indicating its significant effect on concrete's compressive strength. This stems from the fact that the mechanical properties of cement-based mixes depend on hydration pace and extent. The chemical composition of FA plays a crucial role in the strength's development due to its impact on hydration and the formation of strength-contributing products (Moon et al., 2016), (Cho et al., 2019).

Another influential input variable was the concentration of sodium hydroxide (NaOH) in molarity. This parameter significantly impacted the compressive strength of concrete as well. The NaOH concentration influences the dissolution of geopolymer precursor materials, consequently affecting the uniformity and strength of the geopolymer matrix. Higher NaOH concentrations lead to better dissolution, yielding a more uniform and stronger matrix. While fine aggregates, aluminium oxide (Al<sub>2</sub>O<sub>3</sub>), and water also influenced compressive strength, their impact was comparatively lower compared to other input variables.

On the other hand, factors like fine aggregate, the ratio of aluminium oxide to fly ash (AA/FA), and pre-curing conditions exhibited relatively lower PFI values, suggesting their limited impact on compressive strength. Nevertheless, it's worth noting that the PFI index primarily indicates the individual influence of each variable and doesn't capture their combined effects (Naseri et al., 2020), (Asteris et al., 2021).

To delve deeper into the impact of changing individual input variables on concrete's compressive strength, a sensitivity analysis was conducted. For instance, raising the  $\text{SiO}_2$  content in fly ash beyond a certain threshold negatively impacted compressive strength due to the production of unreacted particles. Similarly, within the studied range, increasing the coarse aggregate content,  $\text{SiO}_2$  in fly ash, and NaOH concentration contributed positively to improved compressive strength (Malkawi, 2023), (Moon et al., 2016), (Ghafoor et al., 2021b).

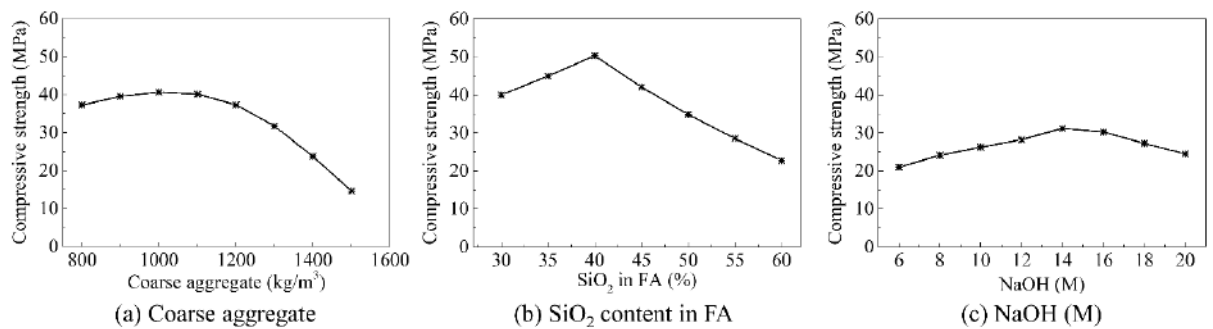


Fig 20: Sensitivity analyses for featured parameters

## 5. CONCLUSIONS AND RECOMENDATIONS

### 5.1.CONCLUSION

The objective of this study was to employ three distinct machine learning algorithms—BPNN, RFR, and KNN—to predict the compressive strength of geopolymer concrete based on fly ash (FA). The dataset was compiled from an extensive literature review, comprising 149 sets of mixing proportions. These sets encompassed 13 variables, such as FA composition ( $\text{SiO}_2$  and  $\text{Al}_2\text{O}_3$ ), coarse and fine aggregate quantities, NaOH and  $\text{Na}_2\text{SiO}_3$  contents and ratio, AA/FA ratio, water content, and pre-curing temperature and duration. Model effectiveness was assessed using metrics like  $R^2$ , MSE, RMSE, and MAE. The models were then compared, and the influence of input parameters on outcomes was analyzed. The conclusions drawn from the results are as follows:

1. The three machine learning algorithms—BPNN, RFR, and KNN—performed well in predicting the compressive strength of FA-based geopolymer concrete. The predictions closely aligned with actual values, with errors generally staying within 20%.
2. The BPNN model exhibited the best performance, achieving an exceptional  $R^2$  of 0.948, outperforming RFR and KNN with  $R^2$  values of 0.927 and 0.911, respectively. The success of BPNN can be attributed to its capacity to grasp intricate data patterns by approximating non-linear functions and adaptively learning from the data, setting it apart from RFR and KNN.

3. Coarse aggregate content, SiO<sub>2</sub> content in FA, and NaOH concentration emerged as the most influential factors with high PFI values, signifying their significant impact on compressive strength. Fine aggregates, Al<sub>2</sub>O<sub>3</sub>, and water had a comparatively smaller influence, whereas AA/FA, FA, and pre-curing conditions had lower PFI values, implying a limited effect. Sensitivity analysis indicated that excessive coarse aggregate, SiO<sub>2</sub>, and NaOH concentration had adverse effects on compressive strength, while moderate increases within the studied range improved it.

## **5.2.RECOMMENDATIONS**

Incorporating machine learning techniques for predicting mechanical properties of FA-based geopolymer concrete offers the potential to streamline and enhance conventional empirical methods. This approach presents a more efficient way to assess concrete strength with untested mixes, reducing the time, effort, and resources required for experimentation. Additionally, it offers a cost-effective and environmentally friendly means of optimizing concrete mixes. Integrating machine learning expedites the progress and adoption of environmentally friendly concrete formulations, such as geopolymers, while minimizing costs and environmental impact. This innovative approach paves the way for a more sustainable and efficient construction industry, promoting eco-friendly development and reducing its carbon footprint.

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